

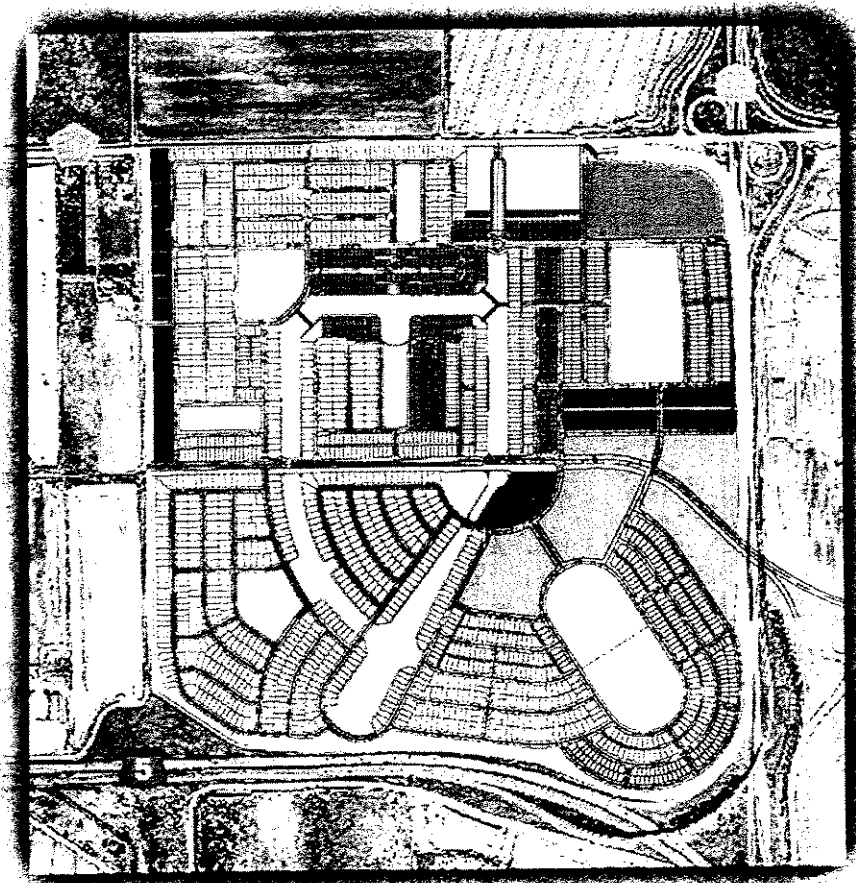
APPENDIX J

GREENBRIAR MASTER DRAINAGE STUDY

Preliminary

Greenbriar

Master Drainage Study



July 2005

Prepared by

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EXECUTIVE SUMMARY

Greenbriar is a proposed residential development of approximately 577 acres located in the County of Sacramento, California. The project has initiated the application process into the City of Sacramento.

The purpose of this report is to present the design of required on-site and off-site drainage facilities.

The proposed development will require an on-site detention basin, outfall structure and gravity storm drain systems which will mitigate the outfall runoff to the existing RD1000 system at a peak discharge value set by RD1000 at 0.1 cfs/acre.



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1. INTRODUCTION

1.1 OVERVIEW

Greenbriar is a proposed mix-use development of approximately 577 acres located in the County of Sacramento, California. There are approximately an additional 50 acres of offsite freeway drainage area draining into the site under existing conditions. The site is bordered by Elkhorn Blvd. along the north boundary, the future Metro Air Park on the west boundary, Interstate-5 along the south, and Highway 99 to the east. See **FIGURES 1, 2, and 3** for the vicinity map, site plan, and the proposed land use plan.

1.2 PURPOSE

This Drainage Study analyzes drainage systems in pre-development conditions and documents the design of the post-development onsite storm drainage system, detention basin, and outfall structure to mitigate storm runoff in post-development conditions.

1.3 PREVIOUS STUDIES

The 2002 Final Metro Air Park Master Drainage Study, prepared by Watermark Engineering, was developed to address drainage associated with the future Metro Airport Park project west of Greenbriar.

The Metro Air Park Off-Site Drainage Improvements Plans by Stantec (formerly known as The Spink Corporation) were developed to improve the drainage facilities from the proposed Metro Air Park pump station outfall to the West Drainage Canal. The data from the improved channel and Interstate 5 crossing was used to analyze the developed Greenbriar downstream condition.

1.4 EXISTING CONDITIONS

The existing site is flat and used for agriculture. The drainage on the site consists of several drainage/irrigations ditches which ultimately convey flows south. The site slopes from westward and southward. Elevations range from approximately 5 to 25 feet mean sea level (MSL).

The existing project site consists of two major watersheds. A narrow eastern shed drains into the existing Natomas Mutual channel, under Highway 99, then southward towards the West Drainage Canal. The western part of the site drains into the Lone Tree Canal, joins runoff from the southern part of the site, and flows under Interstate-5 through three existing 5-ft by 8-ft box culverts towards the West Drainage Canal. The West Drainage Canal drains south and terminates in the Natomas Main Drainage Channel, which is pumped into the Sacramento River.

1.5 FEMA INFORMATION

The most recent Federal Emergency Management Agency's (FEMA) Flood Insurance Study (FIS) Flood Insurance Rate Maps (FIRMs), revised July 06, 1998, shows the site in Zone X, which is designated as outside of a special flood hazard zone. The Flood Insurance Rate Map is displayed in **FIGURE 4**.

1.6 SOILS INFORMATION

Based on the Soil Survey of Sacramento County, the Greenbriar watershed consists primarily of soil classified by the Natural Resources Conservation Service (NRCS) as Hydrologic Soil Group "D". Type "D" soil generally has a high runoff rate and low infiltration rate. (See **FIGURE 5**)

1.7 PROPOSED CONDITIONS ONSITE GRADING

It is intended that the proposed development will not be required to import fill. Earthwork cut and fill volumes were balanced onsite using an Autocad Digital Terrain Model. The model included a soil shrinkage factor of 0.85 to depict the potential shrinkage when the excavated soil is spread and compacted over the site.

1.8 CRITERIA AND METHODOLOGY

1.8.a Existing Conditions

Under Existing Conditions, the 100-year and 10-year storms with 10-day and 24-hour durations were examined. These storms utilized the historic West Drain Canal 100-year 10-day stage hydrograph provided by RD1000 as the downstream boundary condition in the southerly RD1000 Canal. With the Greenbriar site's peak flow discharging into the channel much earlier than the channel's peak flow, the assumption of using the first 24 hours of the 10-day stage hydrograph as the 24-hour storm's boundary condition is conservative. Hydrographs were developed using SACCALC software as documented in the City of Sacramento Drainage Manual, Volume 2 Hydrology Standards (City of Sacramento Standards).

Hydrographs at outlets and places of interest were developed to depict the existing condition. However, no hydraulic model was developed under this condition as there are no significant drainage facilities present at the site.

1.8.b Ultimate Conditions

100-year and 10-year storm hydrographs with 10-day and 24-hour durations were developed for this study reflecting Ultimate Conditions. Hydrology was computed using SACCALC as documented in the City of Sacramento Hydrology Standards.

The 10-year storm was used to analyze the onsite pipe drainage system while the 100-year storm was used to size the detention pond and outfall structure.

To determine detention storage and system outflow, the system was analyzed using an unsteady HEC-RAS model per City of Sacramento Standards. SACCALC output hydrographs were used as input hydrographs in the HEC-RAS model. The outfall structure was modeled such that the peak outflow from the site would be lower than 0.1 cfs per acre or total of 62cfs (set forth by RD1000). The maximum pond water surface was designed to have one foot of freeboard to the top of the pond.

A preliminary on-site storm drain trunk system was designed according to City of Sacramento standards using the Sacramento method for quantifying 10-year peak runoff and hydraulic grade lines. Only major trunk systems

were analyzed and those pipes were designed based on the proposed grading pad and minimum pipe velocity. For the downstream end boundary condition, the 10-year peak stage in the pond from the HEC-RAS model was used.

2. ANALYSIS AND RESULTS

2.1 ANALYSIS OF EXISTING CONDITIONS

The drainage pattern of Greenbriar and its adjacent properties are illustrated in **FIGURE 6**. Part of the existing drainage area as reported in the “Metro Air Park Master Drainage Study,” 2002, prepared by Watermark Engineering, are included. Aerial photographs and topographic maps were utilized to determine the drainage trends of the project site and its surroundings. A site visit determined the approximate size of drainage features.

FIGURE 7 shows the results of the Existing Conditions analysis of the project site. The Existing Conditions SACCALC peak flow is summarized in **Table 1**.

TABLE 1. Existing Conditions 100-Year Peak Flow

Subbasin	Area (ac)	*100yr-10day Peak Flow (cfs)	*100yr-24hr Peak Flow (cfs)
East Basin – E1	448	199	307
West Basin – E2 (Different drainage system)	173	82	134
Metro Offsite – E4	735	253	327
North Offsite – E5	538	252	407
Outflow @ HWY 5 Crossing (Excludes E2)	1721	660	904

*Note that these flows do not reflect onsite and offsite storage that would likely considerably reduce peak flows.

See **APPENDIX A** for watershed flows, land use summaries, watercourse lengths, and rainfall hyetographs in the SACCALC output file.

2.1a Interstate-5 Crossing Drainage Conditions

There are three existing 5-ft by 8-ft box culverts at the I-5 crossing with inverts at approximately 5.5 ft (**REFERENCE NO. 5**). With 904 csf passing through the three existing box culverts, the headwater stage is 13.0 ft (see **APPENDIX C**) which is about a foot lower than the edge of the I-5 pavement. The adjacent low-lying area would likely provide enough storage attenuation to the high water surface and most likely lower headwater elevation.

2.1b Offsite Drainage Conditions

Runoff from the 540 ac offsite watershed north of the project site discharges into the Lone Tree Canal during storms. The drainage ditch parallels the west



property boundary and drains from north to south. The ditch will remain in place under post-development conditions to convey runoff from the offsite watershed. The ditch measures approximately 12 ft wide at bottom and 6 ft deep with 1.5 horizontal to 1 vertical side slopes and 0.0007 ft/ft bottom slope (**REFERENCE NO. 5**). With a 100-year peak flow of 355 cfs in the channel, it is at capacity but not overflowing.

2.2 ANALYSIS OF ULTIMATE CONDITIONS

2.2a Ultimate Detention Basins

Under ultimate conditions, the Greenbriar watershed consists of approximately 620 acres of low, medium, and high-density residential land use, parks, commercial land use and offsite highway drainage.

FIGURE 8 shows the proposed ultimate condition drainage in the context of surrounding drainage features. **FIGURE 9** shows drainage within the project site as developed.

A hydrologic model was developed in SACCALC and its output hydrographs were entered in HEC-RAS around the detention pond.

As shown in **FIGURE 10**, the stormwater detention storage ranges from a water surface of 11.0 ft to 16.0 ft while the permanent pool ranges from a water surface of 3.0 ft to 11.0 ft. The outflow from the detention pond is constrained to 62 cfs with two eight-foot wide Rubicon gates, a 48" reinforced concrete pipe and a flap gate. During the peak stage in the pond, water will overtop the fully closed Rubicon gates. (see **APPENDIX D**)

2.2b Proposed Ultimate Peak Flows, Stages, and Volumes

Required stormwater detention storage volumes and peak stages were developed in the HEC-RAS model. Ultimate conditions runoff and storage results are summarized in **Table 2**.

TABLE 2. Pond Ultimate Condition Results

Peak Parameter Values	100-yr 10-day	100-yr 24-hr	10-yr 24-hr
1. Total Inflow to pond (cfs)	408	912	609
2. Stage (ft)	14.9	14.9	13.5
3. Storage (ac-ft)	330	330	279
4. Outflow (cfs)	62	62	61

See **APPENDIX B** for ultimate condition SACCALC and HEC-RAS input and output files.

2.2c Water Quality

Water quality treatment will be provided in the detention pond per the requirements set out in the "North Natomas Design and Procedures Manual" (**REFERENCE NO.6**). The water quality storage was sized using the Sato Design Curve for Sizing of Water Quality Wet Pond per section 11.6222b of the Sacramento County Hydrology Standards (**REFERENCE NO. 6**). Based



on the 620 ac drainage area and 50% percent imperviousness, Sato volume was determined to be 25 ac-ft. The volume was then multiplied by 1.25 and rounded up for a 35 ac-ft permanent pool requirement. The proposed permanent pool depth of 8.0 ft in the detention pond yields a total wet pool volume of 198 ac-ft, which exceeds the required 35 ac-ft.

2.2d Interstate-5 Crossing Drainage Conditions

The future Metro Air Park plans to improve the Interstate 5 undercrossing by adding two- 78” reinforced concrete pipes. Upstream to downstream flowlines will be 5.6 ft to 5.5 ft, respectively. All of the HEC-RAS models presented in this report were simulated with the proposed pipes.

For the worst-case scenario, a 100-year 24-hr model was simulated without the proposed pipes to reflect that the construction of the two undercrossing pipes did not occur. Differences were only found at the immediate upstream of the crossing where the water surface was about a foot higher and flow was about ten percent more than the existing condition. These changes did not project far enough to influence the hydraulic conditions at the Greenbriar site and in the Lone Tree Channel.

TABLE 3. I-5 Crossing Analysis – Ultimate Conditions

	Proposed Crossing	Existing Crossing
I-5 Crossing Parameters	2-78” RCP & 3-8’x5’ Box	3-8’x5’ Box
Peak Flow (cfs)	376	391
Upstream Stage (ft)	8.9	9.7
Top of Bank (ft)	13	13

2.2e Offsite Drainage Conditions

Under the Ultimate Conditions, Lone Tree Channel was modeled utilizing HEC-RAS, yielding an average freeboard to top of bank of 0.5 ft during the 100-year storm. With the proposed house pad elevations at approximately 18.0 ft along the western site boundary, the freeboard below the house pads is 2.5 ft.

The proposed Metro Air Park 100-year peak pump outflow of 270 cfs was introduced into Lone Tree Channel in the HEC-RAS model during the whole storm duration. This conservative approach generates higher water surfaces than will likely occur.

3. PROPOSED ON-SITE STORM DRAINAGE PIPE SYSTEM

A preliminary design of the on-site storm drainage trunk system was developed consistent with the City of Sacramento requirements. Trunks were sized using the prorated SACCALC watershed flows and the 10-year



detention pond water surface from HEC-RAS output results. The pipes were sized with approximately two feet of freeboard below the proposed grading.

See **FIGURE 9** and **APPENDIX E** for results and calculations.

4. CONCLUSION

Greenbriar can be developed under ultimate conditions with the proposed facilities outlined in this report. The proposed detention basins with permanent water quality features, outlet described and the on-site storm system will adequately convey runoff from the design storm.

The analyses of the adjacent offsite Lone Tree Channel and Interstate 5 crossing proved adequate for severe storms.



5. REFERENCES

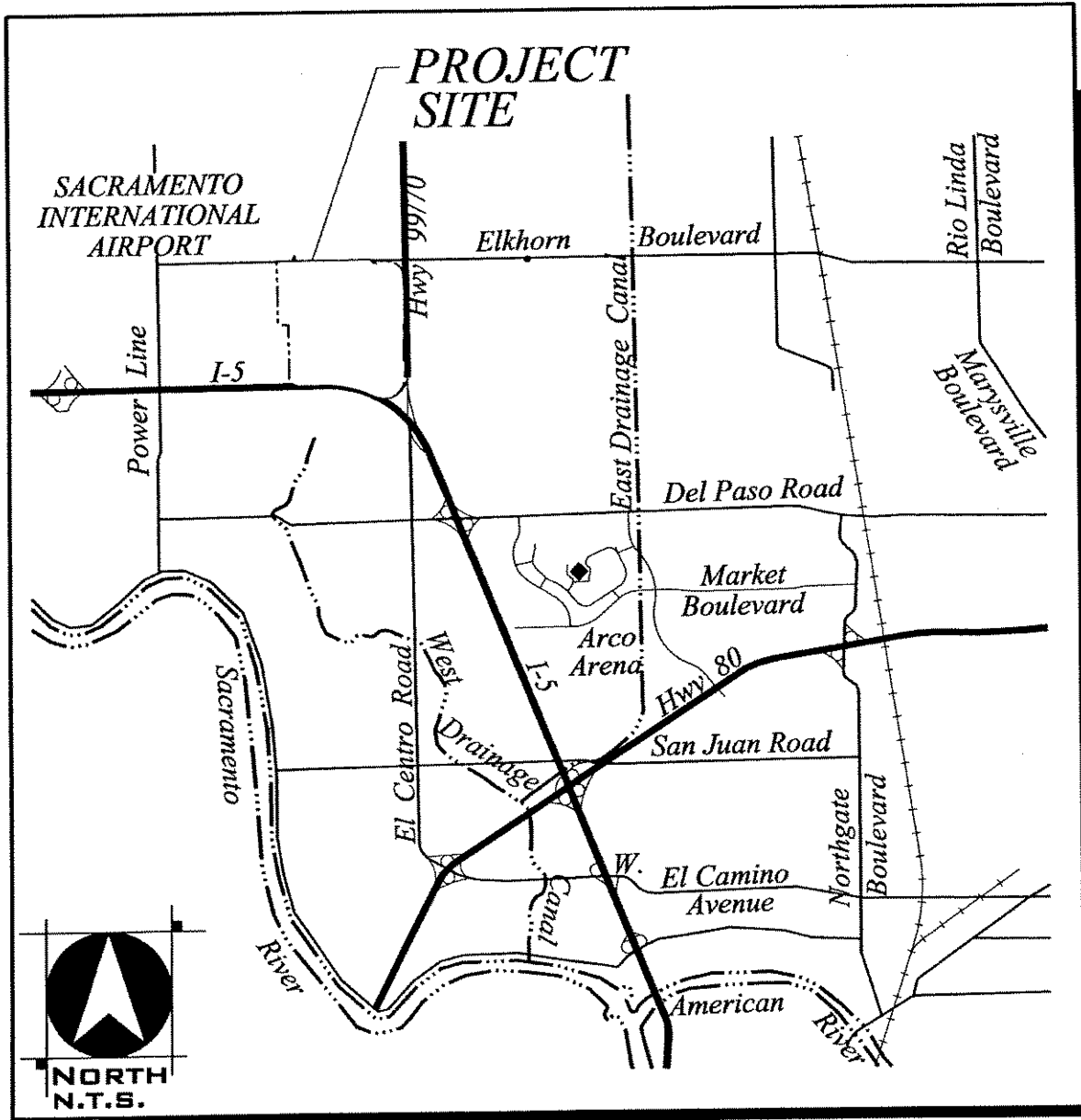
1. Federal Emergency Management Agency, Flood Insurance Study, revised July 06, 1998 and Rate Map for West Sacramento, California, Community Panel Numbers 0602660020F, 060262004E, 0602620045E, Revised July 01, 1998.
2. Watermark Engineering, "Metro Air Park Master Drainage Study," Final 2002.
3. Aerials Express, Digital Aerial Viewer, 2003.
4. USGS Quadrangle Map.
5. Metro Air Park, Offsite Drainage Improvements, The Spin Corporation.
6. North Natomas Drainage Design & Procedures Manual, West Yost & Associates, July 1998.
7. Hydrology Standards, Vol.2 of the Sacramento City/County Drainage Manual, Dec. 1996.

VICINITY MAP

GREENBRIAR

CITY OF SACRAMENTO, CALIFORNIA

JULY, 2005



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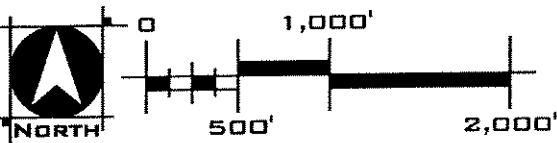
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SITE PLAN
GREENBRIAR
CITY OF SACRAMENTO, CALIFORNIA
JULY, 2005



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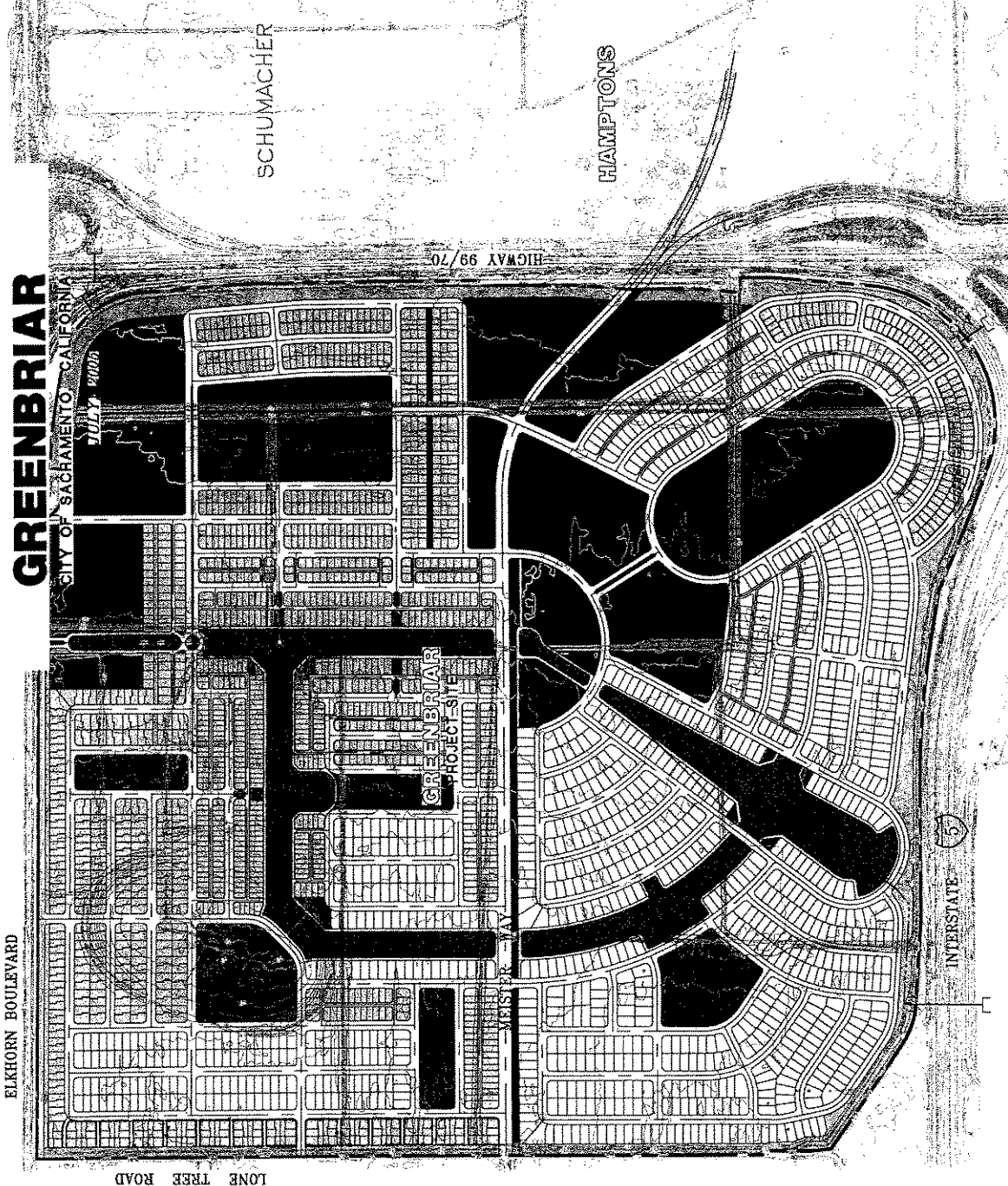
- PROJECT BOUNDARY
- - - EXISTING GROUND CONTOUR
- ▒ PROPOSED DETENTION BASIN



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FIGURE 3

PROPOSED LANDUSE
GREENBRIAR
 CITY OF SACRAMENTO, CALIFORNIA



LEGEND:

- PROJECT BOUNDARY
- EXISTING GROUND CONTOUR
- WATERWAYS AND DRAIN
- DRAIN FLOW DIRECTION
- EXISTING DRAIN CULVERT

LANDUSE LEGEND:

- LDR LOW DENSITY RESIDENTIAL
- MDR MEDIUM DENSITY RESIDENTIAL
- HDR HIGH DENSITY RESIDENTIAL
- PARKS
- LS/OS LANDSCAPE/OPEN SPACE
- VILLAGE COMMERCIAL
- COMMUNITY COMMERCIAL
- DETENTION BASIN
- SCHOOL

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FEMA FLOODPLAIN
GREENBRIAR
 CITY OF SACRAMENTO, CALIFORNIA
 JULY, 2006

FIGURE 4

LEGEND:

--- PROJECT BOUNDARY

Special Flood Hazard Areas Inundated by 100-Year Flood

ZONE A
 No base flood elevations determined.

ZONE AE
 Base flood elevations determined.

ZONE AH
 Flood depths of 1 to 3 feet (usually areas of ponding); base flood elevations determined.

ZONE AD
 Flood depths of 1 to 3 feet, (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

ZONE A99
 To be protected from 100-year flood by Federal flood protection system under construction; no base flood elevations determined.

ZONE AR
 Area of special flood hazard which results from a flood protection system which is an accepted flood protection system which is determined to be in the process of being restored to provide a 100-year or greater level of flood protection.

ZONE V
 Coastal flood with velocity hazard (wave action); no base flood elevations determined.

ZONE VE
 Coastal flood with velocity hazard (wave action); base flood elevations determined.

FLOODWAY AREAS IN ZONE AE

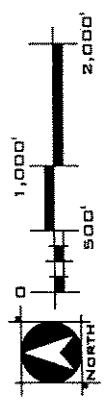
OTHER FLOOD AREAS

ZONE X
 Areas of 500-year flood; areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 100-year flood.

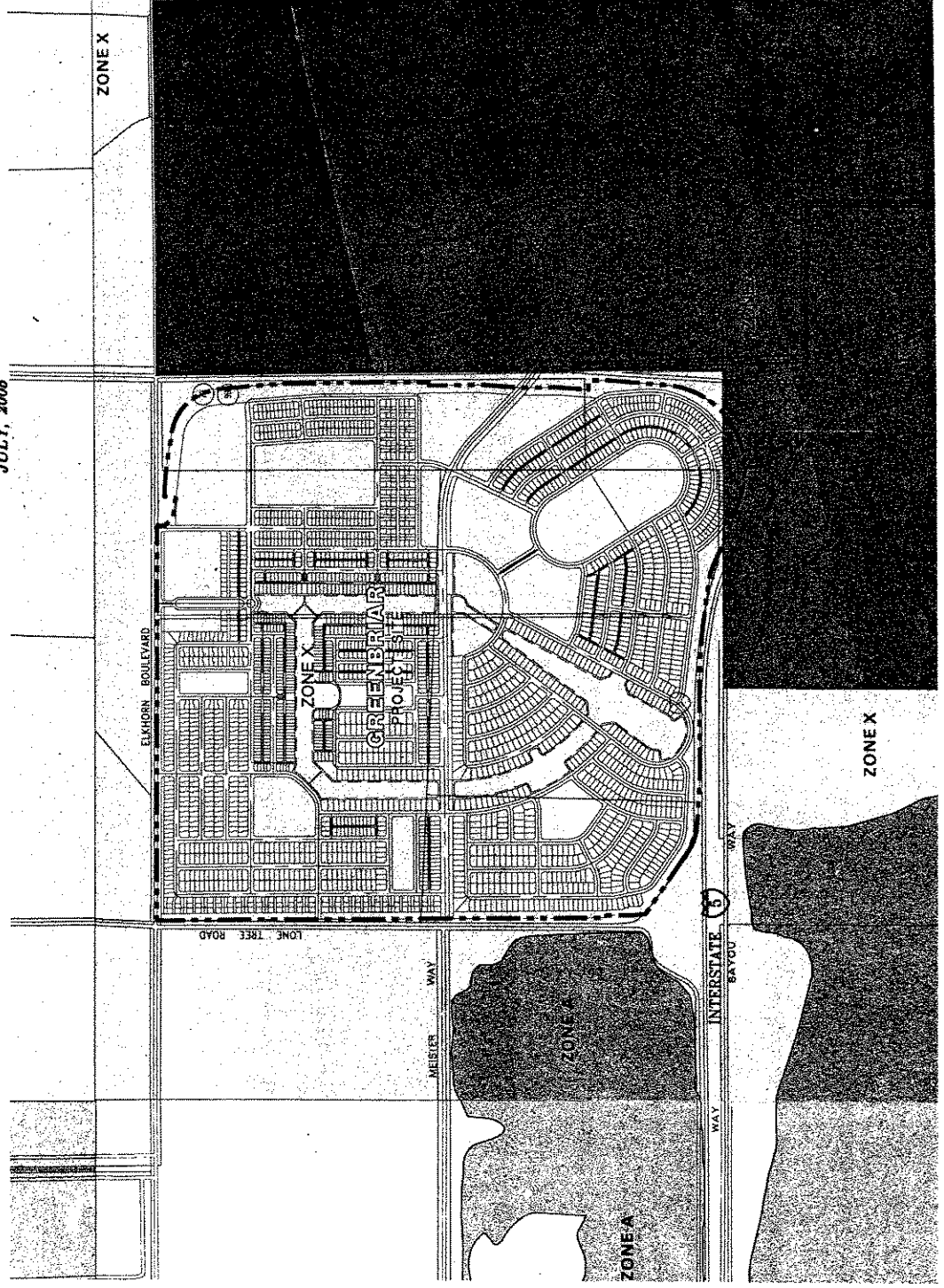
OTHER AREAS
 Areas determined to be outside 500-year flood-plain.

ZONE D
 Areas in which flood hazards are undetermined.

SOURCE:
 1. FEMA FLOOD INSURANCE RATE MAP FOR WEST SACRAMENTO, CALIFORNIA, COMMUNITY PANEL NUMBER 060266020F, 060262004E, 060262004SE, REVISED JULY 06, 1998.



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HYDROLOGIC SOIL MAP GREENBRIAR

CITY OF SACRAMENTO, CALIFORNIA

JULY, 2005



LEGEND:

- PROJECT BOUNDARY
- - - - - EXISTING GROUND CONTOUR
- - - - - EXISTING WATERSHED BOUNDARY
- SOIL GROUP 'D'
- SOIL GROUP 'B'
- SOIL GROUP 'C'
- ① BASIN IDENTIFIER

SOURCES:

1. USGS QUADRANGLE MAP
2. AERIAL PHOTO: AERIALS EXPRESS PHOTO DATE: MARCH 2003
3. SOIL MAP: U.S. DEPARTMENT OF AGRICULTURE CONSERVATION SERVICE, DATED APRIL 1993



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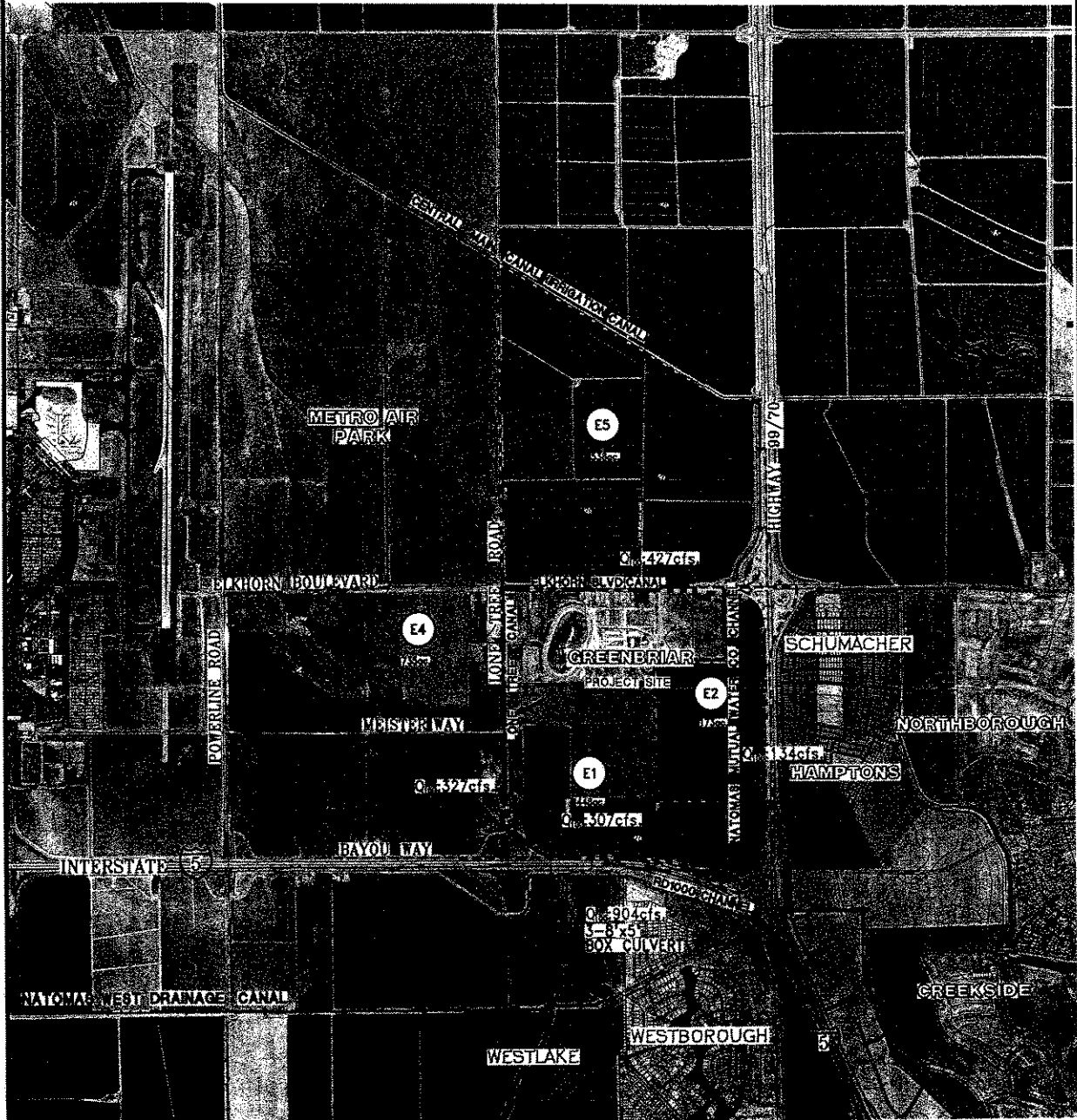
FIGURE 5

OVERALL EXISTING DRAINAGE CONDITIONS

GREENBRIAR

CITY OF SACRAMENTO, CALIFORNIA

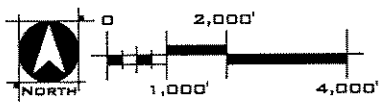
JULY, 2005



LEGEND:

- PROJECT BOUNDARY
- WATERWAYS AND DRAIN
- DRAIN FLOW DIRECTION
- WATERSHED BOUNDARY
- EXISTING GROUND CONTOUR
- OVERLAND RELEASE PATH
- (E5) BASIN IDENTIFIER

- SOURCES:**
1. USGS QUADRANGLE MAP
 2. AERIALS EXPRESS PHOTO DATE: MARCH 2003



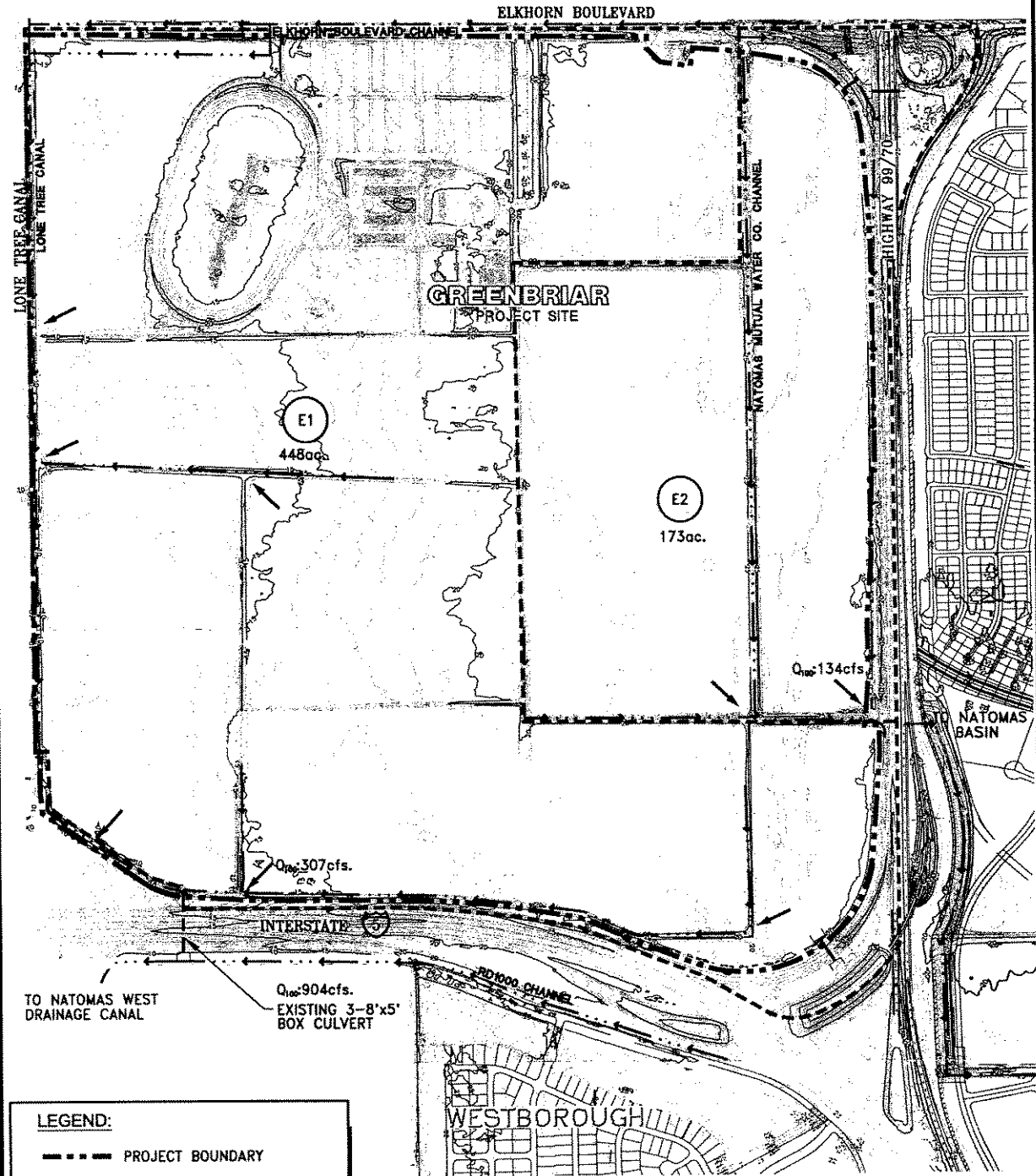
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EXISTING ONSITE DRAINAGE CONDITIONS

GREENBRIAR

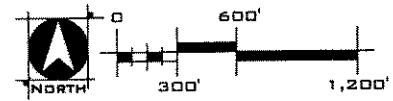
CITY OF SACRAMENTO, CALIFORNIA

JULY, 2005



LEGEND:

- PROJECT BOUNDARY
- - - EXISTING GROUND CONTOUR
- WATERWAYS AND DRAIN
- DRAIN FLOW DIRECTION
- · - · - WATERSHED BOUNDARY
- (E2) BASIN IDENTIFIER




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FIGURE 7

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OVERALL ULTIMATE DRAINAGE CONDITIONS

GREENBRIAR

CITY OF SACRAMENTO, CALIFORNIA

JULY, 2005

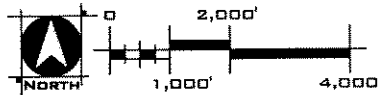


LEGEND:

- PROJECT BOUNDARY
- WATERWAYS AND DRAIN
- DRAIN FLOW DIRECTION
- EXISTING WATERWAYS W/ PROPOSED IMPROVEMENTS
- EXISTING GROUND CONTOUR
- OVERLAND RELEASE PATH
- PROPOSED DETENTION BASIN
- DEVELOPED PROPERTY
- BASIN IDENTIFIER
- PUMP STATION

SOURCES:

1. USGS QUADRANGLE MAP
2. AERIALS EXPRESS PHOTO DATE: MARCH 2003



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FIGURE 8

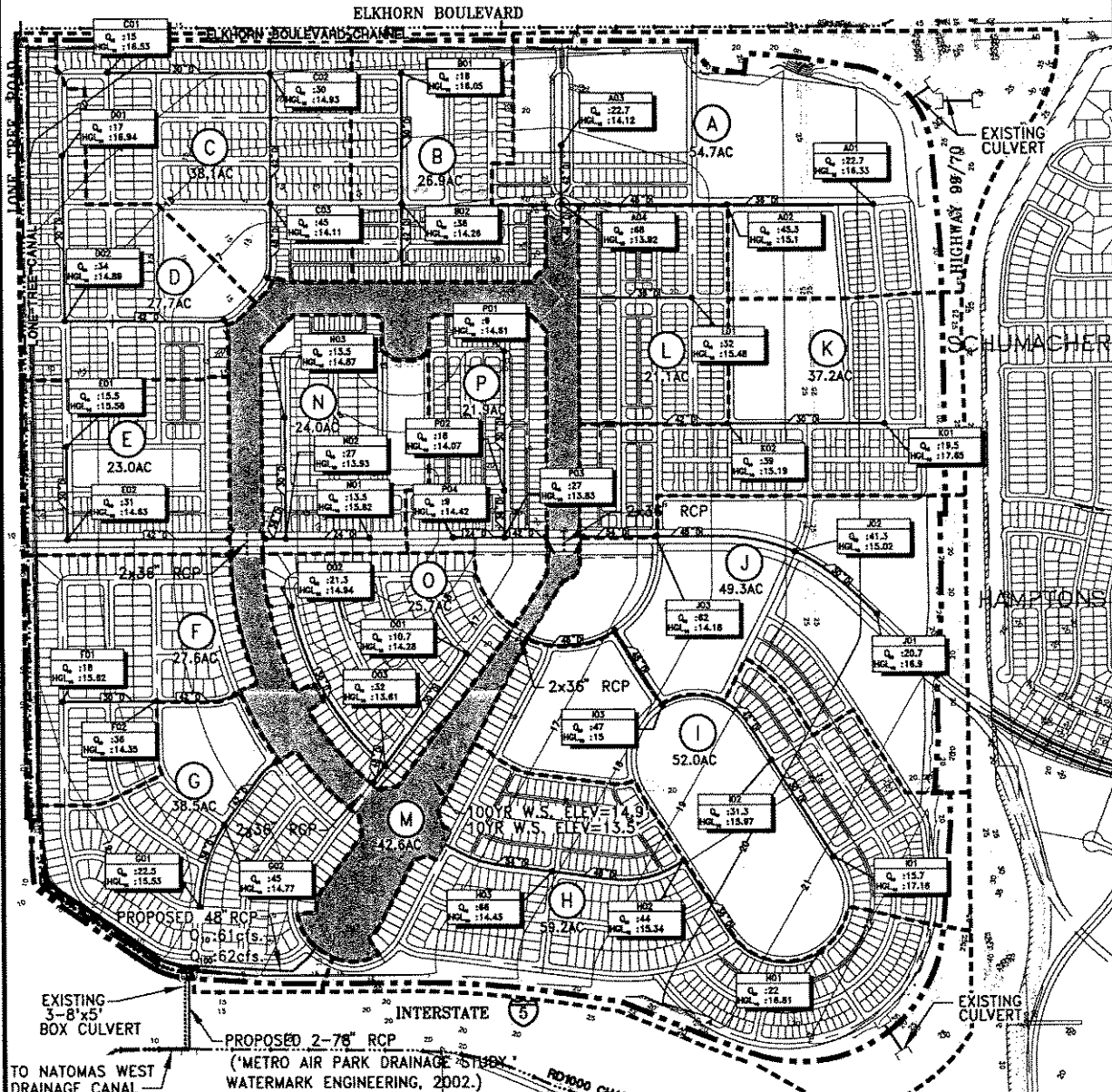
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ULTIMATE ONSITE DRAINAGE CONDITIONS

GREENBRIAR

CITY OF SACRAMENTO, CALIFORNIA

JULY, 2005

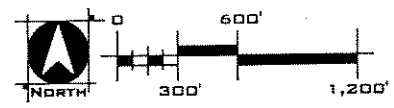


LEGEND:

- PROJECT BOUNDARY
- ~ EXISTING GROUND CONTOUR
- - - PROPOSED GRADE CONTOUR
- WATERWAYS AND DRAIN
- DRAIN FLOW DIRECTION
- EXISTING WATERWAYS W/ PROPOSED IMPROVEMENTS
- PROPOSED DRAIN PIPE
- PROPOSED ONSITE SHED
- PROPOSED DETENTION BASIN
- PROPOSED DRAIN PIPE SIZE
- WATERSHED I.D.
- NODE NUMBER
- 10 yr PEAK FLOW
- 10 yr HGL ELEV.

STAGE-STORAGE DATA	
ELEV. (ft)	STORAGE (cc-ft)
3.0	0.0
4.0	20.0
5.0	42.0
7.0	89.0
11.0	197.0
13.0	261.0
16.0	369.0

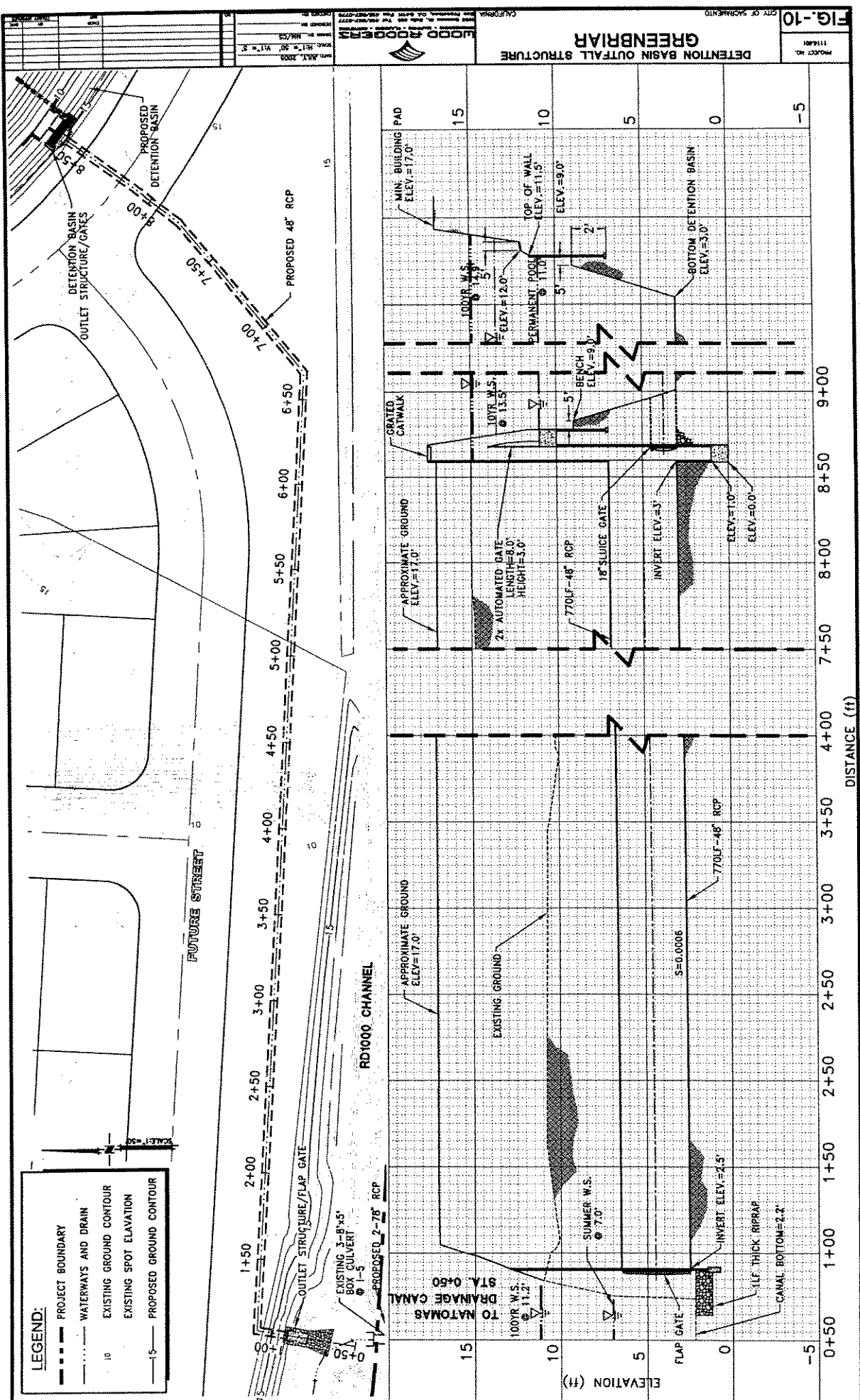
PERMANENT POOL
STORMWATER STORAGE



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FIGURE 9

J:\Jobs\1116-Greenbriar\Forms\Civil\Exhibits\EXH-FIG-9-UL-IMATE_DRAINAGE_COND.dwg 7/26/05 11:42am jcornel



- LEGEND:**
- PROJECT BOUNDARY
 - WATERWAYS AND DRAIN
 - - - EXISTING GROUND CONTOUR
 - 10 EXISTING SPOT ELEVATION
 - 15 PROPOSED GROUND CONTOUR

DETONATION BASIN OUTFALL STRUCTURE GREENBRIAR

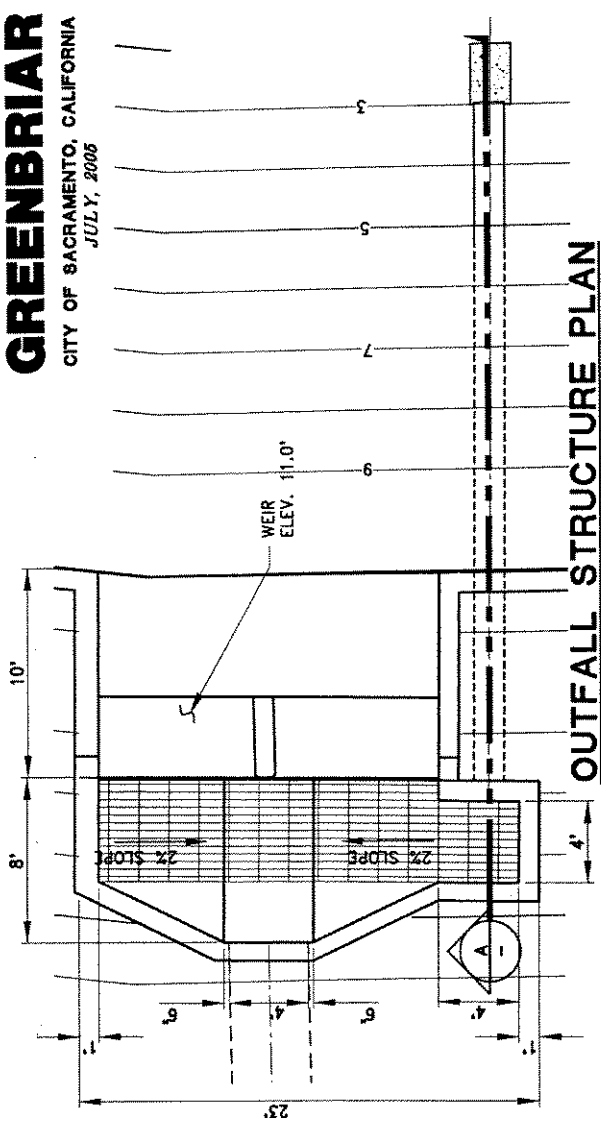
FIG-10

DATE: 07/26/05
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 SCALE: 1"=15'

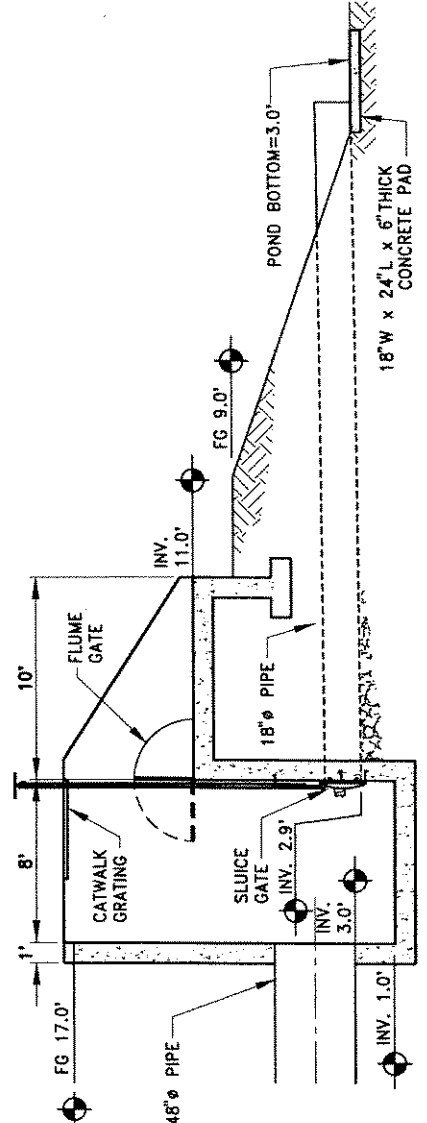
DISTANCE (ft)

ELEVATION (ft)

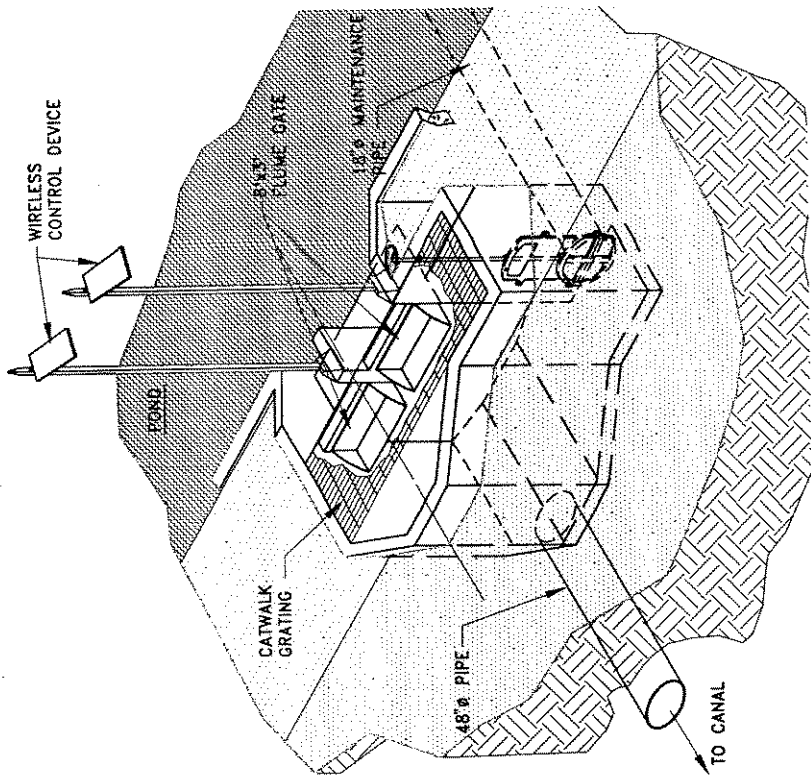
OUTFALL STRUCTURE
GREENBRIAR
 CITY OF SACRAMENTO, CALIFORNIA
 JULY, 2005



OUTFALL STRUCTURE PLAN
 SCALE: 1" = 6"



OUTFALL STRUCTURE SECTION
 SCALE: 1" = 6"



FLUME GATE ISOMETRIC
 NOT TO SCALE

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 1005 Sansome St., Suite 330 Tel 415/627-0777
 San Francisco, CA 94111 Fax 415/627-0778

6. APPENDIX A –
Existing Conditions SacCalc Model

```

INPUT FILENAMR ==>J:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\SACCALC\Existing\sc.dat
OUTPUT FILENAME ==>J:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\SACCALC\Existing\sc.hecout
DSS FILENAME ==>hcalc.dss
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1L) *
* JULY 1998 *
* VERSION 4.1(L) *
*
* RUN DATE 13JUL05 TIME 10:17:29 *
*
*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

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X X XXXXXXX XXXXX X X
X X X X X XX X
X X X X X X X
XXXXXX XXXX X XXXXX X X
X X X X X X X
X X X X X X X
X X XXXXXXX XXXXX XXX XXXX

```

Special version of HEC-1 with Extra-large array
 Modified by David Ford Consulting Engineers (2000)
 Program dimensions:
 Number of hydrograph ordinates: 20000
 Unit hydrograph ordinates: 3000

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL. LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1L INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
* SacCalc, developed by David Ford Consulting Engineers
* File generated 07/13/2005 10:17:28
*
1 ID 10010 & 1010 &10024&1024
* Lag computation for station E1
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
* Developed Undeveloped
* n adj n fract n adj n fract
* -----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.0000 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000
* 0.035 0.0350 0.0000 0.074 0.0740 0.0000
* 0.037 0.0370 0.0000 0.076 0.0760 0.0000
* 0.040 0.0400 0.0000 0.080 0.0800 0.0000
* 0.042 0.0420 0.0000 0.084 0.0840 0.0000
* 0.046 0.0460 0.0000 0.088 0.0880 0.0000
* 0.050 0.0500 0.0000 0.090 0.0900 0.0000
* 0.053 0.0530 0.0000 0.093 0.0930 0.0000
* 0.056 0.0560 0.0000 0.096 0.0960 0.0000
* 0.060 0.0600 0.0000 0.100 0.1000 0.0000
* 0.065 0.0650 0.0000 0.110 0.1100 0.0000
* 0.070 0.0700 0.5000 0.115 0.1150 0.5000
* 0.075 0.0750 0.0000 0.120 0.1200 0.0000
* 0.080 0.0800 0.0000 0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 1.1364, Lc= 0.5682, S= 6.864, n=0.0925
* Resulting lag: 90.9 minutes
* Lag computation for station E2
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
* Developed Undeveloped
* n adj n fract n adj n fract
* -----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.0000 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000

```

```

*   0.035 0.0350 0.0000    0.074 0.0740 0.0000
*   0.037 0.0370 0.0000    0.076 0.0760 0.0000
*   0.040 0.0400 0.0000    0.080 0.0800 0.0000
*   0.042 0.0420 0.0000    0.084 0.0840 0.0000
*   0.046 0.0460 0.0000    0.088 0.0880 0.0000
*   0.050 0.0500 0.0000    0.090 0.0900 0.0000
*   0.053 0.0530 0.0000    0.093 0.0930 0.0000
*   0.056 0.0560 0.0000    0.096 0.0960 0.0000
*   0.060 0.0600 0.0000    0.100 0.1000 0.0000
*   0.065 0.0650 0.0000    0.110 0.1100 0.0000
*   0.070 0.0700 0.5000    0.115 0.1150 0.5000
*   0.075 0.0750 0.0000    0.120 0.1200 0.0000
*   0.080 0.0800 0.0000    0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.7576, Lc= 0.3822, S= 4.224, n=0.0925
* Resulting lag: 75.6 minutes
* Lag computation for station E4
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*   Developed                               Undeveloped
*   n   adj n   fract                       n   adj n   fract
* -----
*   0.030 0.0300 0.0000    0.067 0.0670 0.0000
*   0.031 0.0310 0.0000    0.070 0.0700 0.0000
*   0.032 0.0320 0.0000    0.071 0.0710 0.0000
*   0.033 0.0330 0.0000    0.072 0.0720 0.0000
*   0.034 0.0340 0.0000    0.073 0.0730 0.0000
*   0.035 0.0350 0.0000    0.074 0.0740 0.0000
*   0.037 0.0370 0.0000    0.076 0.0760 0.0000
*   0.040 0.0400 0.0000    0.080 0.0800 0.0000
*   0.042 0.0420 0.0000    0.084 0.0840 0.0000
*   0.046 0.0460 0.0000    0.088 0.0880 0.0000
*   0.050 0.0500 0.0000    0.090 0.0900 0.0000
*   0.053 0.0530 0.0000    0.093 0.0930 0.0000
*   0.056 0.0560 0.0000    0.096 0.0960 0.0000
*   0.060 0.0600 0.0000    0.100 0.1000 0.0000
*   0.065 0.0650 0.0000    0.110 0.1100 0.0000
*   0.070 0.0700 0.5000    0.115 0.1150 0.5000
*   0.075 0.0750 0.0000    0.120 0.1200 0.0000
*   0.080 0.0800 0.0000    0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 2.3674, Lc= 1.1364, S= 2.640, n=0.0925
* Resulting lag: 170.4 minutes
* Lag computation for station E5
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*   Developed                               Undeveloped
*   n   adj n   fract                       n   adj n   fract
* -----
*   0.030 0.0300 0.0000    0.067 0.0670 0.0000
*   0.031 0.0310 0.0000    0.070 0.0700 0.0000
*   0.032 0.0320 0.0000    0.071 0.0710 0.0000
*   0.033 0.0330 0.0000    0.072 0.0720 0.0000
*   0.034 0.0340 0.0000    0.073 0.0730 0.0000
*   0.035 0.0350 0.0000    0.074 0.0740 0.0000
*   0.037 0.0370 0.0000    0.076 0.0760 0.0000
*   0.040 0.0400 0.0000    0.080 0.0800 0.0000
*   0.042 0.0420 0.0000    0.084 0.0840 0.0000
*   0.046 0.0460 0.0000    0.088 0.0880 0.0000
*   0.050 0.0500 0.0000    0.090 0.0900 0.0000
*   0.053 0.0530 0.0000    0.093 0.0930 0.0000
*   0.056 0.0560 0.0000    0.096 0.0960 0.0000
*   0.060 0.0600 0.0000    0.100 0.1000 0.0000
*   0.065 0.0650 0.0000    0.110 0.1100 0.0000
*   0.070 0.0700 0.5000    0.115 0.1150 0.5000
*   0.075 0.0750 0.0000    0.120 0.1200 0.0000
*   0.080 0.0800 0.0000    0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.9470, Lc= 0.3788, S= 5.280, n=0.0925
* Resulting lag: 78.2 minutes
* End of lag computations

```

1

```

* NMIN JXDATE JXTIME      NQ
* HEC-1L INPUT

```

PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
2     IT      1 31DEC99      2400 15592
*     IPRT    IPLT
3     IO      3      0
*
4     KK      E2
*     JXMIN   Time interval for input data
5     IN      60
6     KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 2.959
* multiplier from table 4-4: 0.9808
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional-----Elev-----Areal (adjustments)

```

```

* 5 min      0.2500  0.2500  0.2452
* 10 min     0.3600  0.3600  0.3531
* 15 min     0.4300  0.4300  0.4217
* 30 min     0.5700  0.5700  0.5591
* 1 hour     0.7700  0.7700  0.7552
* 2 hours    1.0400  1.0400  1.0196
* 3 hours    1.2300  1.2300  1.2177
* 6 hours    1.6500  1.6500  1.6335
* 12 hours   2.2500  2.2500  2.2284
* 24 hours   2.9800  2.9800  2.9800
* 36 hours   3.5400  3.5400  3.5046
* 2 days     3.9500  3.9500  3.9500
* 3 days     4.6500  4.6500  4.6500
* 5 days     5.7600  5.7600  5.7024
* 10 days    7.5400  7.5400  7.4646

```

```

* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646

```

```

7  PB 0
8  PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
9  PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
10 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
11 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
12 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
13 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
14 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
15 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
16 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
17 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
18 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
19 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
20 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
21 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
22 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
23 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
24 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
25 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
26 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493

```

HEC-1L INPUT

1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
27  UI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
28  UI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
29  UI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
30  UI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
31  UI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
32  BA 0.2703
* STRIL CNSTL RTIMP
33  LU 0.20 0.070 2.000
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 75.6 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 76.07659
* Volume of runoff (Step 4) V= 7.268403
34  UI 0.96 1.93 2.89 3.94 5.33 6.71 8.10 9.51 10.96 12.40
35  UI 13.85 15.38 16.96 18.54 20.12 21.40 22.59 23.78 24.97 27.12
36  UI 29.29 31.46 33.55 35.29 37.02 38.75 40.64 42.80 44.95 47.11
37  UI 49.32 51.56 53.81 56.05 58.48 60.97 63.46 65.94 68.46 70.97
38  UI 73.49 76.08 79.08 82.08 85.08 88.16 91.39 94.61 97.84 100.90
39  UI 103.84 106.78 109.72 110.99 111.70 112.41 113.12 113.53 113.92 114.31
40  UI 114.59 114.20 113.81 113.42 112.92 112.21 111.50 110.80 109.22 106.89
41  UI 104.55 102.22 99.83 97.41 95.00 92.59 90.15 87.71 85.27 82.86
42  UI 80.72 78.58 76.44 74.56 73.22 71.87 70.53 69.20 67.87 66.55
43  UI 65.22 63.72 62.16 60.59 59.02 58.00 57.04 56.07 55.10 54.09
44  UI 53.08 52.07 51.05 50.01 48.97 47.93 47.02 46.24 45.45 44.67
45  UI 43.88 43.08 42.28 41.48 40.76 40.05 39.34 38.64 37.91 37.19
46  UI 36.47 35.80 35.31 34.81 34.31 33.77 33.18 32.60 32.01 31.50
47  UI 31.03 30.57 30.10 29.62 29.14 28.66 28.18 27.74 27.30 26.87
48  UI 26.45 26.09 25.72 25.36 25.02 24.71 24.39 24.07 23.72 23.34
49  UI 22.96 22.59 22.27 21.97 21.67 21.37 21.10 20.83 20.56 20.29
50  UI 20.01 19.74 19.47 19.23 19.04 18.84 18.65 18.38 18.08 17.78
51  UI 17.48 17.29 17.12 16.96 16.79 16.52 16.25 15.98 15.74 15.62
52  UI 15.50 15.38 15.22 15.01 14.80 14.58 14.45 14.36 14.27 14.18
53  UI 14.04 13.89 13.74 13.59 13.48 13.37 13.27 13.16 13.02 12.89
54  UI 12.75 12.61 12.48 12.34 12.21 12.08 11.96 11.84 11.72 11.62
55  UI 11.53 11.44 11.35 11.23 11.11 11.09 10.87 10.77 10.66 10.56
56  UI 10.46 10.39 10.31 10.24 10.14 10.04 9.93 9.83 9.72 9.62
57  UI 9.51 9.40 9.34 9.28 9.22 9.15 9.05 8.94 8.84 8.75
58  UI 8.69 8.63 8.57 8.48 8.38 8.27 8.17 8.11 8.06 8.02
59  UI 7.97 7.87 7.76 7.66 7.56 7.50 7.44 7.38 7.31 7.24
60  UI 7.16 7.09 7.03 7.00 6.97 6.94 6.87 6.78 6.69 6.60
61  UI 6.54 6.48 6.42 6.36 6.30 6.23 6.17 6.11 6.05 5.99
62  UI 5.93 5.88 5.84 5.79 5.75 5.70 5.65 5.61 5.56 5.51
63  UI 5.45 5.39 5.33 5.28 5.24 5.19 5.15 5.10 5.06 5.01
64  UI 4.96 4.92 4.87 4.83 4.78 4.74 4.69 4.65 4.60 4.56
65  UI 4.51 4.47 4.42 4.38 4.33 4.29 4.26 4.23 4.20 4.16
66  UI 4.10 4.04 3.98 3.94 3.90 3.87 3.84 3.80 3.76 3.71
67  UI 3.67 3.64 3.61 3.58 3.55 3.52 3.49 3.46 3.43 3.40
68  UI 3.37 3.34 3.31 3.27 3.24 3.21 3.18 3.15 3.12 3.09

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

70	UI	2.81	2.80	2.78	2.77	2.75	2.73	2.70	2.67	2.64	2.62
71	UI	2.61	2.59	2.57	2.54	2.51	2.48	2.45	2.42	2.39	2.36
72	UI	2.34	2.33	2.31	2.30	2.28	2.27	2.25	2.24	2.21	2.18
73	UI	2.14	2.12	2.10	2.09	2.07	2.05	2.02	1.99	1.96	1.94
74	UI	1.92	1.91	1.89	1.88	1.86	1.85	1.83	1.82	1.80	1.79
75	UI	1.77	1.76	1.74	1.73	1.71	1.70	1.68	1.67	1.65	1.64
76	UI	1.62	1.61	1.59	1.58	1.56	1.55	1.53	1.52	1.50	1.49
77	UI	1.47	1.46	1.44	1.43	1.41	1.40	1.38	1.38	1.36	1.38
78	UI	1.38	1.36	1.35	1.33	1.32	1.30	1.29	1.27	1.26	1.24
79	UI	1.23	1.21	1.20	1.20	1.20	1.20				

80 ZW C=FLOW F=010YR-10DY A=GREENBRI

81 KK E1
 * JKMIN Time interval for input data
 82 IN 60
 83 KM

* Design storm construction details
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 2.959
 * multiplier from table 4-4: 0.9808
 * Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration-----Regional-----Elev-----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2452
 * 10 min 0.3600 0.3600 0.3531
 * 15 min 0.4300 0.4300 0.4217
 * 30 min 0.5700 0.5700 0.5591
 * 1 hour 0.7700 0.7700 0.7552
 * 2 hours 1.0400 1.0400 1.0196
 * 3 hours 1.2300 1.2300 1.2177
 * 6 hours 1.6500 1.6500 1.6335
 * 12 hours 2.2500 2.2500 2.2284
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646

84 PB 0

85	PI	0.0224	0.0821	0.2015	0.1120	0.0373	0.0224	0.0075	0.0000	0.0000	0.0000
86	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
87	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
88	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224
89	PI	0.0373	0.0523	0.0672	0.0970	0.2239	0.1418	0.0746	0.0597	0.0448	0.0373
90	PI	0.0299	0.0224	0.0149	0.0075	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
91	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
92	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
93	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
94	PI	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224	0.0299	0.0373	0.0448	0.0523
95	PI	0.0672	0.1120	0.3956	0.1642	0.0746	0.0597	0.0448	0.0373	0.0299	0.0224

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

96	PI	0.0224	0.0149	0.0149	0.0075	0.0075	0.0000	0.0000	0.0000	0.0000	0.0000
97	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075
98	PI	0.0149	0.0149	0.0149	0.0224	0.0373	0.0448	0.0523	0.0672	0.0746	0.0821
99	PI	0.0970	0.1045	0.1194	0.1269	0.1344	0.1418	0.1568	0.1120	0.0896	0.0672
100	PI	0.2314	0.2911	0.5001	0.2463	0.0373	0.0224	0.0149	0.0075	0.0075	0.0075
101	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
102	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
103	PI	0.0075	0.0149	0.0224	0.0299	0.0373	0.0523	0.0672	0.0970	0.2911	0.1493
104	PI	0.0746	0.0597	0.0523	0.0448	0.0373	0.0299	0.0224	0.0149	0.0075	0.0000
105	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
106	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
107	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075
108	PI	0.0149	0.0373	0.0523	0.0746	0.2165	0.1194	0.0597	0.0448	0.0299	0.0149

* Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TAREA subbasin area (sq mi)
 109 BA 0.7000
 * STRTL CNSTL RTIMP
 110 LU 0.20 0.070 2.800
 * Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 90.9 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 91.39346
 * Volume of runoff (Step 4) V= 18.822222

111	UI	1.73	3.46	5.19	6.92	8.98	11.47	13.95	16.44	18.93	21.51
112	UI	24.11	26.70	29.30	31.97	34.80	37.64	40.48	43.32	45.65	47.79
113	UI	49.93	52.06	54.47	58.36	62.25	66.15	70.04	73.48	76.59	79.70
114	UI	82.81	85.93	89.79	93.66	97.53	101.39	105.33	109.36	113.39	117.42

115	UI	121.45	125.86	130.32	134.78	139.24	143.72	148.24	152.75	157.27	161.78
116	UI	166.93	172.32	177.70	183.08	188.53	194.31	200.10	205.89	211.67	217.15
117	UI	222.43	227.70	232.97	238.15	243.42	248.69	254.96	261.23	267.50	273.77
118	UI	245.65	246.35	247.06	246.51	245.81	245.11	244.40	243.52	242.25	240.98
119	UI	239.71	238.44	234.99	230.80	226.61	222.42	218.20	213.87	209.55	205.22
120	UI	200.89	196.54	192.15	187.77	183.39	179.03	175.19	171.35	167.51	163.67
121	UI	160.50	158.10	155.69	153.28	150.88	148.49	146.11	143.73	141.36	138.83
122	UI	136.02	133.21	130.40	127.58	125.59	123.86	122.13	120.40	118.65	116.84
123	UI	115.03	113.22	111.41	109.56	107.70	105.83	103.97	102.12	100.72	99.31
124	UI	97.90	96.50	95.08	93.65	92.21	90.78	89.35	88.06	86.79	85.52
125	UI	84.25	82.97	81.67	80.37	79.07	77.78	76.79	75.90	75.01	74.11
126	UI	73.19	72.13	71.08	70.02	68.97	68.05	67.21	66.37	65.54	64.70
127	UI	63.83	62.97	62.10	61.24	60.41	59.63	58.84	58.06	57.27	56.61
128	UI	55.96	55.32	54.67	54.05	53.48	52.91	52.34	51.77	51.12	50.45
129	UI	49.77	49.09	48.45	47.91	47.37	46.82	46.28	45.78	45.29	44.80
130	UI	44.32	43.83	43.34	42.86	42.37	41.88	41.47	41.11	40.76	40.41
131	UI	40.06	39.53	38.99	38.45	37.91	37.46	37.16	36.86	36.56	36.27
132	UI	35.82	35.33	34.85	34.36	33.93	33.72	33.50	33.28	33.07	32.74
133	UI	32.37	31.99	31.61	31.25	31.09	30.92	30.76	30.60	30.38	30.11
134	UI	29.84	29.57	29.30	29.11	28.92	28.73	28.54	28.33	28.09	27.84
135	UI	27.60	27.36	27.11	26.87	26.63	26.38	26.15	25.93	25.71	25.50
136	UI	25.28	25.10	24.94	24.78	24.61	24.45	24.23	24.01	23.80	23.58
137	UI	23.38	23.19	23.00	22.81	22.62	22.48	22.35	22.21	22.08	21.92
138	UI	21.73	21.54	21.35	21.16	20.98	20.79	20.60	20.41	20.24	20.13

HEC-1L INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
139	UI	20.02	19.91	19.81	19.64	19.45	19.26	19.07	18.89	18.79	18.68
140	UI	18.57	18.46	18.31	18.12	17.93	17.74	17.55	17.47	17.39	17.31
141	UI	17.22	17.10	16.91	16.72	16.53	16.34	16.22	16.11	16.00	15.90
142	UI	15.78	15.65	15.51	15.38	15.24	15.16	15.11	15.05	15.00	14.93
143	UI	14.77	14.61	14.45	14.28	14.15	14.04	13.93	13.83	13.72	13.61
144	UI	13.50	13.39	13.29	13.18	13.07	12.96	12.85	12.74	12.66	12.58
145	UI	12.50	12.42	12.33	12.25	12.17	12.09	12.01	11.91	11.80	11.69
146	UI	11.59	11.48	11.40	11.32	11.24	11.16	11.08	10.99	10.91	10.83
147	UI	10.75	10.67	10.59	10.51	10.43	10.35	10.26	10.18	10.10	10.02
148	UI	9.94	9.86	9.78	9.70	9.62	9.53	9.45	9.37	9.29	9.23
149	UI	9.17	9.12	9.07	9.00	8.90	8.79	8.68	8.57	8.49	8.44
150	UI	8.39	8.33	8.28	8.20	8.12	8.04	7.95	7.88	7.83	7.78
151	UI	7.72	7.67	7.61	7.56	7.51	7.45	7.40	7.34	7.29	7.24
152	UI	7.18	7.13	7.07	7.02	6.97	6.91	6.86	6.80	6.75	6.69
153	UI	6.64	6.59	6.53	6.48	6.42	6.37	6.32	6.26	6.21	6.17
154	UI	6.14	6.11	6.09	6.06	6.03	6.00	5.98	5.95	5.91	5.86
155	UI	5.81	5.75	5.70	5.66	5.64	5.61	5.58	5.55	5.50	5.44
156	UI	5.39	5.33	5.28	5.23	5.27	5.12	5.07	5.04	5.01	4.98
157	UI	4.96	4.93	4.90	4.88	4.85	4.82	4.77	4.72	4.66	4.61
158	UI	4.56	4.54	4.51	4.48	4.46	4.41	4.35	4.30	4.25	4.20
159	UI	4.17	4.14	4.12	4.09	4.06	4.03	4.01	3.98	3.95	3.93
160	UI	3.90	3.87	3.84	3.82	3.79	3.76	3.74	3.71	3.68	3.66
161	UI	3.63	3.60	3.57	3.55	3.52	3.49	3.47	3.44	3.41	3.39
162	UI	3.36	3.33	3.30	3.28	3.25	3.22	3.20	3.17	3.14	3.11
163	UI	3.09	3.06	3.03	3.01	2.98	2.97	2.97	2.97	2.97	2.97
164	UI	2.94	2.91	2.89	2.86	2.83	2.81	2.78	2.75	2.72	2.70
165	UI	2.67	2.64	2.62	2.59	2.59	2.59	2.59	2.59		
166	ZW	C=FLOW F=010YR-10DY A=GREENBRI									

167 KK E4
 * JXMIN Time interval for input data
 168 IN 60
 169 KM Part of future Metro Air Park
 *
 * Design storm construction details
 *
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 2.959
 * multiplier from table 4-4: 0.9808
 *
 * Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration----Regional----Elev----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2452
 * 10 min 0.3600 0.3600 0.3531
 * 15 min 0.4300 0.4300 0.4217
 * 30 min 0.5700 0.5700 0.5591
 * 1 hour 0.7700 0.7700 0.7552
 * 2 hours 1.0400 1.0400 1.0196
 * 3 hours 1.2300 1.2300 1.2177
 * 6 hours 1.6500 1.6500 1.6335
 * 12 hours 2.2500 2.2500 2.2284
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646
 HEC-1L INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
170	PB	0									
171	PI	0.0224	0.0821	0.2015	0.1120	0.0373	0.0224	0.0075	0.0000	0.0000	0.0000
172	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

252	UI	15.79	15.70	15.62	15.53	15.44	15.37	15.34	15.30	15.26	15.22
253	UI	15.18	15.15	15.11	15.07	14.98	14.89	14.81	14.72	14.63	14.54
254	UI	14.45	14.36	14.29	14.24	14.19	14.14	14.09	14.03	13.98	13.93
255	UI	13.88	13.82	13.76	13.69	13.63	13.57	13.50	13.44	13.38	13.33
256	UI	13.30	13.27	13.25	13.22	13.20	13.17	13.15	13.12	13.06	12.98
257	UI	12.91	12.83	12.75	12.68	12.60	12.53	12.45	12.40	12.35	12.30
258	UI	12.25	12.20	12.15	12.10	12.05	12.00	11.95	11.90	11.85	11.80
259	UI	11.74	11.69	11.64	11.59	11.54	11.49	11.44	11.39	11.34	11.29
260	UI	11.24	11.19	11.14	11.11	11.07	11.03	10.99	10.95	10.91	10.88
261	UI	10.84	10.80	10.76	10.72	10.69	10.65	10.61	10.57	10.53	10.49
262	UI	10.44	10.39	10.34	10.29	10.24	10.19	10.13	10.08	10.05	10.01
263	UI	9.97	9.93	9.89	9.86	9.82	9.78	9.74	9.70	9.66	9.63
264	UI	9.59	9.55	9.51	9.47	9.44	9.40	9.36	9.32	9.28	9.25
265	UI	9.21	9.17	9.13	9.09	9.06	9.02	8.98	8.94	8.90	8.87
266	UI	8.83	8.79	8.75	8.71	8.68	8.64	8.60	8.56	8.52	8.49
267	UI	8.45	8.41	8.37	8.33	8.30	8.26	8.22	8.18	8.14	8.11

HEC-1L INPUT

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
268	UI	8.09	8.06	8.04	8.01	7.99	7.96	7.94	7.91	7.86	7.81
269	UI	7.76	7.70	7.65	7.60	7.55	7.50	7.47	7.44	7.41	7.39
270	UI	7.36	7.34	7.31	7.29	7.26	7.22	7.19	7.15	7.11	7.07
271	UI	7.03	7.00	6.96	6.93	6.90	6.87	6.85	6.82	6.80	6.77
272	UI	6.75	6.72	6.70	6.67	6.65	6.62	6.60	6.57	6.54	6.52
273	UI	6.49	6.47	6.44	6.42	6.39	6.37	6.34	6.32	6.29	6.27
274	UI	6.24	6.22	6.19	6.16	6.14	6.11	6.09	6.06	6.04	6.01
275	UI	5.99	5.96	5.94	5.91	5.89	5.86	5.83	5.81	5.78	5.76
276	UI	5.73	5.71	5.68	5.66	5.63	5.61	5.58	5.56	5.53	5.50
277	UI	5.48	5.45	5.43	5.41	5.40	5.39	5.37	5.36	5.35	5.34
278	UI	5.32	5.31	5.30	5.28	5.27	5.26	5.25	5.23	5.22	5.21
279	UI	5.19	5.16	5.14	5.11	5.09	5.06	5.04	5.01	4.99	4.97
280	UI	4.96	4.95	4.94	4.92	4.91	4.90	4.88	4.87	4.84	4.82
281	UI	4.79	4.76	4.74	4.71	4.69	4.66	4.64	4.61	4.59	4.56
282	UI	4.54	4.51	4.49	4.46	4.44	4.43	4.41	4.40	4.39	4.38
283	UI	4.36	4.35	4.34	4.33	4.31	4.30	4.29	4.27	4.26	4.25
284	UI	4.24	4.22	4.20	4.17	4.14	4.12	4.09	4.07	4.04	4.02
285	UI	4.00	3.99	3.98	3.96	3.95	3.94	3.93	3.91	3.90	3.87
286	UI	3.85	3.82	3.80	3.77	3.75	3.72	3.69	3.68	3.66	3.65
287	UI	3.64	3.63	3.61	3.60	3.59	3.58	3.56	3.55	3.54	3.53
288	UI	3.51	3.50	3.49	3.47	3.46	3.45	3.44	3.42	3.41	3.40
289	UI	3.39	3.37	3.36	3.35	3.33	3.32	3.31	3.30	3.28	3.27
290	UI	3.26	3.25	3.23	3.22	3.21	3.20	3.18	3.17	3.16	3.14
291	UI	3.13	3.12	3.11	3.09	3.08	3.07	3.06	3.04	3.03	3.02
292	UI	3.01	2.99	2.98	2.97	2.95	2.94	2.93	2.92	2.90	2.89
293	UI	2.88	2.87	2.85	2.84	2.83	2.81	2.80	2.79	2.78	2.76
294	UI	2.75	2.74	2.73	2.71	2.70	2.69	2.68	2.66	2.65	2.64
295	UI	2.62	2.61	2.60	2.60	2.60	2.60	2.60	2.60	2.60	2.60
296	UI	2.60	2.59	2.58	2.57	2.56	2.54	2.53	2.52	2.50	2.49
297	UI	2.48	2.47	2.45	2.44	2.43	2.42	2.40	2.39	2.38	2.37
298	UI	2.35	2.34	2.33	2.31	2.30	2.29	2.28	2.28	2.28	2.28
299	UI	2.28	2.28	2.28	2.28	2.28					
300	ZW	C=FLOW F=010YR-10DY A=GREENBRI									

KK E5
* JXMIN Time interval for input data

IN 60
KM E5

* Design storm construction details

* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 2.959
* multiplier from table 4-4: 0.9808

* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional-----Elev-----Areal (adjustments)

* 5 min	0.2500	0.2500	0.2452
* 10 min	0.3600	0.3600	0.3531
* 15 min	0.4300	0.4300	0.4217
* 30 min	0.5700	0.5700	0.5591
* 1 hour	0.7700	0.7700	0.7552
* 2 hours	1.0400	1.0400	1.0196
* 3 hours	1.2300	1.2300	1.2177
* 6 hours	1.6500	1.6500	1.6335
* 12 hours	2.2500	2.2500	2.2284
* 24 hours	2.9800	2.9800	2.9800
* 36 hours	3.5400	3.5400	3.5046
* 2 days	3.9500	3.9500	3.9500
* 3 days	4.6500	4.6500	4.6500
* 5 days	5.7600	5.7600	5.7024
* 10 days	7.5400	7.5400	7.4646

* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646

HEC-1L INPUT

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
304	PB	0									
305	PI	0.0224	0.0821	0.2015	0.1120	0.0373	0.0224	0.0075	0.0000	0.0000	0.0000
306	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
307	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
308	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224	
309	PI	0.0373	0.0523	0.0672	0.0970	0.2239	0.1418	0.0746	0.0597	0.0448	0.0373


```

310      PI  0.0299  0.0224  0.0149  0.0075  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
311      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
312      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
313      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
314      PI  0.0000  0.0000  0.0000  0.0075  0.0149  0.0224  0.0299  0.0373  0.0448  0.0523  0.0597
315      PI  0.0672  0.1120  0.1956  0.1642  0.0746  0.0597  0.0448  0.0373  0.0299  0.0224  0.0149
316      PI  0.0224  0.0149  0.0149  0.0075  0.0075  0.0075  0.0000  0.0000  0.0000  0.0000  0.0000
317      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
318      PI  0.0149  0.0149  0.0149  0.0224  0.0373  0.0448  0.0523  0.0672  0.0746  0.0821  0.0896
319      PI  0.0970  0.1045  0.1194  0.1269  0.1344  0.1418  0.1568  0.1120  0.0896  0.0672  0.0448
320      PI  0.2314  0.2911  0.5001  0.2463  0.0373  0.0224  0.0149  0.0075  0.0075  0.0075  0.0075
321      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
322      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
323      PI  0.0075  0.0149  0.0224  0.0299  0.0373  0.0523  0.0672  0.0970  0.2911  0.1493  0.0000
324      PI  0.0746  0.0597  0.0523  0.0448  0.0373  0.0299  0.0224  0.0149  0.0075  0.0000  0.0000
325      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
326      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000
327      PI  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0000  0.0075
328      PI  0.0149  0.0373  0.0523  0.0746  0.2165  0.1194  0.0597  0.0448  0.0299  0.0149  0.0000

```

```

* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)

```

```

329      BA  0.8406
* SRTSL CNSTL RTIMP
330      LU  0.20  0.070  2.000

```

```

* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 78.2 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 78.67958
* Volume of runoff (Step 4) V= 22.603472

```

```

331      UI  2.80  5.61  8.41  11.30  15.33  19.36  23.39  27.45  31.65  35.86
332      UI  40.06  44.35  48.95  53.55  58.15  62.45  65.91  69.37  72.83  77.24
333      UI  83.55  89.86  96.17  101.97  107.01  112.05  117.09  122.69  128.96  135.22
334      UI  141.49  147.90  154.42  160.95  167.48  174.43  181.66  188.89  196.11  203.40
335      UI  210.72  218.04  225.35  233.69  242.41  251.13  259.85  269.09  278.46  287.84
336      UI  297.22  305.88  314.42  322.97  331.51  334.06  336.12  338.18  340.24  341.39
337      UI  342.53  343.67  344.68  343.54  342.40  341.26  340.01  337.95  335.89  333.83
338      UI  330.88  324.09  317.30  310.51  303.66  296.65  289.64  282.63  275.59  268.49
339      UI  261.39  254.29  247.53  241.31  235.09  228.87  223.70  219.80  215.90  212.00
340      UI  208.12  204.27  200.41  196.55  192.29  187.73  183.17  178.62  175.20  172.40
341      UI  169.59  166.79  163.89  160.95  158.02  155.08  152.08  149.06  146.03  143.01
342      UI  140.62  138.34  136.06  133.78  131.46  129.14  126.82  124.50  122.43  120.37
343      UI  118.31  116.25  114.15  112.05  109.94  107.91  106.47  105.02  103.58  102.08
344      UI  100.37  98.67  96.96  95.33  93.98  92.62  91.26  89.89  88.48  87.08
345      UI  85.68  84.33  83.06  81.79  80.52  79.34  78.29  77.24  76.19  75.20
346      UI  74.28  73.36  72.44  71.42  70.33  69.23  68.14  67.18  66.30  65.43

```

HEC-1L INPUT

PAGE 11

1

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
347      UI  64.55  63.74  62.95  62.16  61.37  60.58  59.79  59.00  58.22  57.61
348      UI  57.04  56.47  55.90  55.06  54.18  53.30  52.43  51.93  51.45  50.97
349      UI  50.48  49.69  48.90  48.11  47.37  47.02  46.66  46.31  45.92  45.31
350      UI  44.69  44.08  43.55  43.29  43.02  42.76  42.44  42.01  41.57  41.13
351      UI  40.74  40.43  40.13  39.82  39.47  39.08  38.69  38.29  37.90  37.50
352      UI  37.11  36.71  36.34  35.99  35.64  35.29  35.00  34.73  34.47  34.21
353      UI  33.89  33.53  33.18  32.83  32.52  32.21  31.90  31.60  31.36  31.14
354      UI  30.92  30.71  30.41  30.10  29.79  29.49  29.18  28.87  28.57  28.26
355      UI  28.09  27.91  27.74  27.55  27.24  26.94  26.63  26.34  26.17  25.99
356      UI  25.82  25.61  25.31  25.00  24.69  24.44  24.31  24.18  24.04  23.85
357      UI  23.54  23.24  22.93  22.68  22.50  22.33  22.15  21.96  21.74  21.52
358      UI  21.30  21.15  21.07  20.98  20.89  20.69  20.43  20.17  19.90  19.70
359      UI  19.53  19.35  19.18  19.00  18.83  18.65  18.48  18.30  18.12  17.95
360      UI  17.77  17.64  17.51  17.37  17.24  17.11  16.98  16.85  16.72  16.54
361      UI  16.37  16.19  16.02  15.89  15.76  15.63  15.49  15.36  15.23  15.10
362      UI  14.97  14.84  14.71  14.57  14.44  14.31  14.18  14.05  13.92  13.78
363      UI  13.65  13.52  13.39  13.26  13.13  13.00  12.89  12.80  12.71  12.62
364      UI  12.49  12.31  12.14  11.96  11.84  11.75  11.66  11.58  11.46  11.33
365      UI  11.20  11.07  10.97  10.88  10.79  10.70  10.62  10.53  10.44  10.35
366      UI  10.27  10.18  10.09  10.00  9.91  9.83  9.74  9.65  9.56  9.48
367      UI  9.39  9.30  9.21  9.13  9.04  8.95  8.86  8.78  8.69  8.61
368      UI  8.57  8.52  8.48  8.43  8.39  8.35  8.30  8.24  8.16  8.07
369      UI  7.98  7.91  7.87  7.82  7.78  7.72  7.63  7.54  7.45  7.36
370      UI  7.28  7.19  7.10  7.04  7.00  6.95  6.91  6.87  6.82  6.78
371      UI  6.73  6.66  6.57  6.48  6.40  6.34  6.30  6.26  6.21  6.13
372      UI  6.04  5.95  5.87  5.82  5.78  5.73  5.69  5.64  5.60  5.56
373      UI  5.51  5.47  5.43  5.38  5.34  5.29  5.25  5.21  5.16  5.12
374      UI  5.07  5.03  4.99  4.94  4.90  4.86  4.81  4.77  4.72  4.68
375      UI  4.64  4.59  4.55  4.51  4.46  4.42  4.37  4.33  4.29  4.24
376      UI  4.20  4.15  4.14  4.14  4.14  4.14  4.11  4.06  4.02  3.98
377      UI  3.93  3.89  3.85  3.80  3.76  3.71  3.67  3.63  3.62  3.62
378      UI  3.62  3.62
379      ZW  C=FLOW F=010YR-10DY A=GREENBRI

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380      EK  ISCROS
381      KM
382      HC  3
383      ZW  C=FLOW
384      ZZ

```

```

* FLOOD HYDROGRAPH PACKAGE (HEC-1L) *
*   JULY 1998 *
*   VERSION 4.1(L) *
*   *
* RUN DATE 13JUL05 TIME 10:17:29 *
*   *
*****

```

```

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*   *
*****

```

10010 & 1010 & 10024&1024

```

3 IO      OUTPUT CONTROL VARIABLES
          IPRINT      3  PRINT CONTROL
          IPLOT       0  PLOT CONTROL
          QSCAL       0.  HYDROGRAPH PLOT SCALE

```

```

IT      HYDROGRAPH TIME DATA
        NMIN         1  MINUTES IN COMPUTATION INTERVAL
        IDATE        31DEC99  STARTING DATE
        ITIME        0000  STARTING TIME
        NQ           15592  NUMBER OF HYDROGRAPH ORDINATES
        NDDATE       11JAN 0  ENDING DATE
        NDTIME       1951  ENDING TIME
        ICENT        19  CENTURY MARK

        COMPUTATION INTERVAL 0.02 HOURS
        TOTAL TIME BASE 259.85 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRES-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

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*****
*   *
4 KK * E2 *
*   *
*****

```

```

5 IN      TIME DATA FOR INPUT TIME SERIES
          JXMIN       60  TIME INTERVAL IN MINUTES
          JXDATE      31DEC99  STARTING DATE
          JXTIME      2400  STARTING TIME

```

SUBBASIN RUNOFF DATA

```

32 BA     SUBBASIN CHARACTERISTICS
          TAREA,     0.27  SUBBASIN AREA

```

PRECIPITATION DATA

```

7 PB      STORM      7.46  BASIN TOTAL PRECIPITATION

```

```

33 LU     UNIFORM LOSS RATE
          STRTL      0.20  INITIAL LOSS
          CNSTL      0.07  UNIFORM LOSS RATE
          RTIMP      2.00  PERCENT IMPERVIOUS AREA

```

```

32 UI     INPUT UNITGRAPH, 456 ORDINATES, VOLUME = 1.00

```

1.0	1.9	2.9	3.9	5.3	6.7	8.1	9.5	11.0	12.4
13.9	15.4	17.0	18.5	20.1	21.4	22.6	23.8	25.0	27.1
29.3	31.5	33.5	35.3	37.0	38.8	40.6	42.8	45.0	47.1
49.3	51.6	53.8	56.0	58.5	61.0	63.5	65.9	68.5	71.0
73.5	76.1	79.1	82.1	85.1	88.2	91.4	94.6	97.8	100.9
103.8	106.8	109.7	111.0	111.7	112.4	113.1	113.5	113.9	114.3
114.6	114.2	113.8	113.4	112.9	112.2	111.5	110.8	109.2	106.9
104.6	102.2	99.8	97.4	95.0	92.6	90.2	87.7	85.3	82.9
80.7	78.6	76.4	74.6	73.2	71.9	70.5	69.2	67.9	66.6
65.2	63.7	62.2	60.6	59.0	58.0	57.0	56.1	55.1	54.1
53.1	52.1	51.0	50.0	49.0	47.9	47.0	46.2	45.5	44.7
43.9	43.1	42.3	41.5	40.8	40.0	39.3	38.6	37.9	37.2
36.5	35.8	35.3	34.8	34.3	33.8	33.2	32.6	32.0	31.5
31.0	30.6	30.1	29.6	29.1	28.7	28.2	27.7	27.3	26.9
26.5	26.1	25.7	25.4	25.0	24.7	24.4	24.1	23.7	23.3
23.0	22.6	22.3	22.0	21.7	21.4	21.1	20.8	20.6	20.3
20.0	19.7	19.5	19.2	19.0	18.8	18.6	18.4	18.1	17.8
17.5	17.3	17.1	17.0	16.8	16.5	16.3	16.0	15.7	15.6
15.5	15.4	15.2	15.0	14.8	14.6	14.4	14.4	14.3	14.2
14.0	13.9	13.7	13.6	13.5	13.4	13.3	13.2	13.0	12.9
12.8	12.6	12.5	12.3	12.2	12.1	12.0	11.8	11.7	11.6
11.5	11.4	11.4	11.2	11.1	11.0	10.9	10.8	10.7	10.6
10.5	10.4	10.3	10.2	10.1	10.0	9.9	9.8	9.7	9.6

9.5	9.4	9.3	9.3	9.2	9.1	9.1	8.9	8.8	8.8
8.7	8.6	8.6	8.5	8.4	8.3	8.2	8.1	8.1	8.0
8.0	7.9	7.8	7.7	7.6	7.5	7.4	7.4	7.3	7.2
7.2	7.1	7.0	7.0	7.0	6.9	6.9	6.8	6.7	6.6
6.5	6.5	6.4	6.4	6.3	6.2	6.2	6.1	6.1	6.0
5.9	5.9	5.8	5.8	5.8	5.7	5.7	5.6	5.6	5.5
5.4	5.4	5.3	5.3	5.2	5.2	5.2	5.1	5.1	5.0
5.0	4.9	4.9	4.8	4.8	4.7	4.7	4.7	4.6	4.6
4.5	4.5	4.4	4.4	4.3	4.3	4.3	4.2	4.2	4.2
4.1	4.0	4.0	3.9	3.9	3.9	3.8	3.8	3.8	3.7
3.7	3.6	3.6	3.6	3.5	3.5	3.5	3.5	3.4	3.4
3.4	3.3	3.3	3.3	3.2	3.2	3.2	3.2	3.1	3.1
3.1	3.0	3.0	3.0	2.9	2.9	2.9	2.9	2.8	2.8
2.8	2.8	2.8	2.8	2.8	2.7	2.7	2.7	2.6	2.6
2.6	2.6	2.6	2.5	2.5	2.5	2.5	2.4	2.4	2.4
2.3	2.3	2.3	2.3	2.3	2.3	2.3	2.2	2.2	2.2
2.1	2.1	2.1	2.1	2.1	2.0	2.0	2.0	2.0	1.9
1.9	1.9	1.9	1.9	1.9	1.9	1.8	1.8	1.8	1.8
1.8	1.8	1.7	1.7	1.7	1.7	1.7	1.7	1.6	1.6
1.6	1.6	1.6	1.6	1.6	1.5	1.5	1.5	1.5	1.5
1.5	1.5	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4
1.4	1.4	1.4	1.3	1.3	1.3	1.3	1.3	1.3	1.2
1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2

*** *** *** *** ***

HYDROGRAPH AT STATION E2

TOTAL RAINFALL = 7.46, TOTAL LOSS = 4.55, TOTAL EXCESS = 2.92

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	259.85-HR
52.	153.58	28.	11.	5.	2.
		0.949 (INCHES)	1.484	1.969	2.916
		14. (AC-PT)	21.	28.	42.

CUMULATIVE AREA = 0.27 SQ MI

* *
81 KK * E1 *
* *

82 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

109 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.70 SUBBASIN AREA

PRECIPITATION DATA

84 PB STORM 7.46 BASIN TOTAL PRECIPITATION

110 LU UNIFORM LOSS RATE

 STRYL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 2.00 PERCENT IMPERVIOUS AREA

109 UI INPUT UNITGRAPH, 548 ORDINATES, VOLUME = 1.00

1.7	3.5	5.2	6.9	9.0	11.5	13.9	16.4	18.9	21.5
24.1	26.7	29.3	32.0	34.8	37.6	40.5	43.3	45.7	47.8
49.9	52.1	54.5	58.4	62.3	66.2	70.0	73.5	76.6	79.7
82.8	85.9	89.8	93.7	97.5	101.4	105.3	109.4	113.4	117.4
121.4	125.9	130.3	134.8	139.2	143.7	148.2	152.8	157.3	161.8
166.9	172.3	177.7	183.1	188.5	194.3	200.1	205.9	211.7	217.1
222.4	227.7	233.0	238.1	239.4	240.7	242.0	243.2	244.2	244.9
245.6	246.4	247.1	246.5	245.8	245.1	244.4	243.5	242.3	241.0
239.7	238.4	235.0	230.8	226.6	222.4	218.2	213.9	209.6	205.2
200.9	196.5	192.1	187.8	183.4	179.0	175.2	171.4	167.5	163.7
160.5	158.1	155.7	153.3	150.9	148.5	146.1	143.7	141.4	138.8
136.0	133.2	130.4	127.6	125.6	123.9	122.1	120.4	118.7	116.8
115.0	113.2	111.4	109.6	107.7	105.8	104.0	102.1	100.7	99.3
97.9	96.5	95.1	93.7	92.2	90.8	89.3	88.1	86.8	85.5
84.3	83.0	81.7	80.4	79.1	77.8	76.8	75.9	75.0	74.1
73.2	72.1	71.1	70.0	69.0	68.1	67.2	66.4	65.5	64.7
63.8	63.0	62.1	61.2	60.4	59.6	58.8	58.1	57.3	56.6
56.0	55.3	54.7	54.0	53.5	52.9	52.3	51.8	51.1	50.5
49.8	49.1	48.5	47.9	47.4	46.8	46.3	45.8	45.3	44.8
44.3	43.8	43.3	42.9	42.4	41.9	41.5	41.1	40.8	40.4

40.1	39.5	39.0	38.5	37.9	37.5	37.2	36.9	36.6	36.3
35.8	35.3	34.8	34.4	33.9	33.7	33.5	33.3	33.1	32.7
32.4	32.0	31.6	31.3	31.1	30.9	30.8	30.6	30.4	30.1
29.8	29.6	29.3	29.1	28.9	28.7	28.5	28.3	28.1	27.8
27.6	27.4	27.1	26.9	26.6	26.4	26.1	25.9	25.7	25.5
25.3	25.1	24.9	24.8	24.6	24.5	24.2	24.0	23.8	23.6
23.4	23.2	23.0	22.8	22.6	22.5	22.4	22.2	22.1	21.9
21.7	21.5	21.4	21.2	21.0	20.8	20.6	20.4	20.2	20.1
20.0	19.9	19.8	19.6	19.5	19.3	19.1	18.9	18.8	18.7
18.6	18.5	18.3	18.1	17.9	17.7	17.5	17.5	17.4	17.3
17.2	17.1	16.9	16.7	16.5	16.3	16.2	16.1	16.0	15.9
15.8	15.6	15.5	15.4	15.2	15.2	15.1	15.1	15.0	14.9
14.8	14.6	14.4	14.3	14.1	14.0	13.9	13.8	13.7	13.6
13.5	13.4	13.3	13.2	13.1	13.0	12.9	12.7	12.7	12.6
12.5	12.4	12.3	12.3	12.2	12.1	12.0	11.9	11.8	11.7
11.6	11.5	11.4	11.3	11.2	11.2	11.1	11.0	10.9	10.8
10.8	10.7	10.6	10.5	10.4	10.4	10.3	10.2	10.1	10.0
9.9	9.9	9.8	9.7	9.6	9.5	9.4	9.4	9.3	9.2
9.2	9.1	9.1	9.0	8.9	8.8	8.7	8.6	8.5	8.4
8.4	8.3	8.3	8.2	8.1	8.0	7.9	7.9	7.8	7.8
7.7	7.7	7.6	7.6	7.5	7.4	7.4	7.3	7.3	7.2
7.2	7.1	7.1	7.0	7.0	6.9	6.9	6.8	6.8	6.7
6.6	6.6	6.5	6.5	6.4	6.4	6.3	6.3	6.2	6.2
6.1	6.1	6.1	6.1	6.0	6.0	6.0	5.9	5.9	5.9
5.8	5.8	5.7	5.7	5.6	5.6	5.6	5.5	5.5	5.4
5.4	5.3	5.3	5.2	5.2	5.1	5.1	5.0	5.0	5.0
5.0	4.9	4.9	4.9	4.8	4.8	4.8	4.7	4.7	4.6
4.6	4.5	4.5	4.5	4.5	4.4	4.3	4.3	4.3	4.2
4.2	4.1	4.1	4.1	4.1	4.0	4.0	4.0	4.0	3.9
3.9	3.9	3.8	3.8	3.8	3.8	3.7	3.7	3.7	3.7
3.6	3.6	3.6	3.5	3.5	3.5	3.5	3.4	3.4	3.4
3.4	3.3	3.3	3.3	3.3	3.2	3.2	3.2	3.1	3.1
3.1	3.1	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
2.9	2.9	2.9	2.9	2.8	2.8	2.8	2.8	2.7	2.7
2.7	2.6	2.6	2.6	2.6	2.6	2.6	2.6		

*** *** *** *** ***

HYDROGRAPH AT STATION E1

TOTAL RAINFALL = 7.46, TOTAL LOSS = 4.55, TOTAL EXCESS = 2.92

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	259.85-HR	
127.	153.78	70.	28.	12.	5.	
		(INCHES)	0.929	1.483	1.969	2.916
		(AC-FT)	35.	55.	74.	109.

CUMULATIVE AREA = 0.70 SQ MI

167 KK *
 * E4 *
 * *

Part of future Metro Air Park

168 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

195 BA SUBBASIN CHARACTERISTICS
 TAREA, 1.15 SUBBASIN AREA

PRECIPITATION DATA

170 PB STORM 7.46 BASIN TOTAL PRECIPITATION

196 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 2.00 PERCENT IMPERVIOUS AREA

195 UI	INPUT UNITGRAPH, 1025 ORDINATES,		VOLUME = 1.00							
	0.8	1.6	2.4	3.3	4.1	4.9	5.7	6.5	7.5	8.6
	9.8	11.0	12.1	13.3	14.5	15.6	16.8	18.0	19.2	20.5
	21.7	22.9	24.1	25.3	26.5	27.8	29.1	30.5	31.8	33.1
	34.5	35.8	37.1	38.5	39.5	40.5	41.5	42.5	43.5	44.5
	45.5	46.5	47.8	49.6	51.4	53.2	55.1	56.9	58.7	60.5
	62.4	63.9	65.4	66.8	68.3	69.8	71.2	72.7	74.1	75.7
	77.5	79.3	81.1	82.9	84.7	86.5	88.3	90.2	92.0	93.9

95.8	97.7	99.6	101.5	103.4	105.3	107.2	109.3	111.3	113.4
115.5	117.6	119.7	121.8	123.9	126.0	128.1	130.3	132.4	134.5
136.6	138.7	140.8	143.0	145.5	148.0	150.5	153.1	155.6	158.1
160.6	163.1	165.8	168.5	171.2	173.9	176.6	179.3	182.0	184.8
187.5	190.0	192.4	194.9	197.4	199.9	202.3	204.8	207.3	209.1
209.7	210.3	210.9	211.5	212.1	212.7	213.3	213.9	214.3	214.6
214.9	215.2	215.6	215.9	216.2	216.6	216.7	216.4	216.1	215.7
215.4	215.1	214.7	214.4	214.1	213.6	213.0	212.4	211.8	211.2
210.6	210.0	209.4	208.6	206.6	204.6	202.7	200.7	198.7	196.8
194.8	192.8	190.8	188.8	186.8	184.7	182.7	180.7	178.6	176.6
174.6	172.5	170.5	168.4	166.4	164.3	162.3	160.2	158.1	156.2
154.4	152.6	150.8	149.0	147.2	145.4	143.6	141.8	140.7	139.6
138.4	137.3	136.2	135.0	133.9	132.8	131.6	130.5	129.4	128.3
127.2	126.1	124.9	123.8	122.7	121.4	120.1	118.8	117.5	116.2
114.8	113.5	112.2	111.1	110.2	109.4	108.6	107.8	107.0	106.2
105.4	104.6	103.7	102.9	102.0	101.2	100.3	99.5	98.6	97.8
96.9	95.0	95.2	94.3	93.4	92.5	91.7	90.8	89.9	89.2
88.5	87.9	87.2	86.5	85.9	85.2	84.6	83.9	83.2	82.6
81.9	81.2	80.5	79.9	79.2	78.5	77.9	77.3	76.7	76.1
75.5	74.9	74.3	73.7	73.1	72.5	71.9	71.3	70.7	70.1
69.5	68.9	68.3	67.8	67.3	66.9	66.5	66.1	65.7	65.2
64.8	64.4	63.9	63.4	62.9	62.4	61.9	61.4	60.9	60.4
60.0	59.6	59.2	58.8	58.4	58.0	57.6	57.2	56.8	56.4
56.0	55.6	55.2	54.8	54.4	54.0	53.6	53.2	52.8	52.5
52.1	51.7	51.4	51.0	50.6	50.3	50.0	49.6	49.3	49.0
48.7	48.4	48.1	47.8	47.5	47.3	47.0	46.7	46.5	46.2
45.9	45.7	45.4	45.1	44.8	44.5	44.1	43.8	43.5	43.2
42.9	42.6	42.3	42.1	41.8	41.5	41.3	41.0	40.8	40.5
40.3	40.1	39.8	39.6	39.4	39.2	38.9	38.7	38.5	38.2
38.0	37.8	37.6	37.3	37.1	36.9	36.6	36.5	36.3	36.1
36.0	35.8	35.6	35.5	35.3	35.1	34.9	34.6	34.4	34.1
33.9	33.6	33.3	33.1	32.9	32.8	32.6	32.5	32.3	32.2
32.1	31.9	31.8	31.6	31.3	31.1	30.9	30.6	30.4	30.2
30.0	29.8	29.7	29.6	29.5	29.4	29.3	29.2	29.1	29.0
28.8	28.6	28.5	28.3	28.1	27.9	27.7	27.6	27.4	27.3
27.3	27.2	27.1	27.0	27.0	26.9	26.8	26.7	26.6	26.4
26.3	26.2	26.0	25.9	25.8	25.7	25.6	25.5	25.4	25.3
25.2	25.1	25.1	25.0	24.9	24.8	24.6	24.5	24.4	24.3
24.2	24.1	24.0	23.8	23.7	23.6	23.5	23.4	23.3	23.1
23.0	22.9	22.8	22.7	22.6	22.5	22.4	22.4	22.2	22.1
22.0	22.0	21.9	21.8	21.7	21.7	21.6	21.5	21.4	21.3
21.2	21.1	21.0	20.9	20.8	20.7	20.6	20.5	20.4	20.3
20.3	20.2	20.1	20.0	19.9	19.8	19.8	19.7	19.6	19.6
19.5	19.4	19.4	19.3	19.2	19.1	19.0	19.0	18.9	18.8
18.7	18.6	18.5	18.4	18.3	18.3	18.2	18.1	18.0	17.9
17.8	17.7	17.7	17.6	17.6	17.5	17.5	17.4	17.4	17.3
17.2	17.1	17.1	17.0	16.9	16.8	16.7	16.6	16.6	16.5
16.5	16.4	16.4	16.3	16.3	16.2	16.1	16.1	16.0	15.9
15.8	15.7	15.6	15.5	15.4	15.4	15.3	15.3	15.3	15.2
15.2	15.1	15.1	15.1	15.0	14.9	14.8	14.7	14.6	14.5
14.4	14.4	14.3	14.2	14.2	14.1	14.1	14.0	14.0	13.9
13.9	13.8	13.8	13.7	13.6	13.6	13.5	13.4	13.4	13.3
13.3	13.3	13.3	13.2	13.2	13.2	13.1	13.1	13.1	13.0
12.9	12.8	12.8	12.7	12.6	12.5	12.4	12.4	12.4	12.3
12.3	12.2	12.1	12.1	12.1	12.0	11.9	11.9	11.9	11.8
11.7	11.7	11.6	11.6	11.5	11.5	11.4	11.4	11.3	11.3
11.2	11.2	11.1	11.1	11.1	11.0	11.0	10.9	10.9	10.9
10.8	10.8	10.8	10.7	10.7	10.6	10.6	10.6	10.5	10.5
10.4	10.4	10.3	10.3	10.2	10.2	10.1	10.1	10.1	10.0
10.0	9.9	9.9	9.9	9.8	9.8	9.7	9.7	9.7	9.6
9.6	9.6	9.5	9.5	9.4	9.4	9.4	9.3	9.3	9.3
9.2	9.2	9.1	9.1	9.1	9.0	9.0	8.9	8.9	8.9
8.8	8.8	8.8	8.7	8.7	8.6	8.6	8.6	8.5	8.5
8.4	8.4	8.4	8.3	8.3	8.3	8.2	8.2	8.1	8.1
8.1	8.1	8.0	8.0	8.0	8.0	7.9	7.9	7.9	7.8
7.8	7.7	7.7	7.6	7.6	7.5	7.5	7.4	7.4	7.4
7.4	7.3	7.3	7.3	7.3	7.2	7.2	7.2	7.1	7.1
7.0	7.0	7.0	6.9	6.9	6.9	6.8	6.8	6.8	6.8
6.8	6.7	6.7	6.7	6.7	6.6	6.6	6.6	6.5	6.5
6.5	6.5	6.4	6.4	6.4	6.4	6.3	6.3	6.3	6.3
6.2	6.2	6.2	6.2	6.1	6.1	6.1	6.1	6.0	6.0
6.0	6.0	5.9	5.9	5.9	5.9	5.8	5.8	5.8	5.8
5.7	5.7	5.7	5.7	5.6	5.6	5.6	5.6	5.5	5.5
5.5	5.4	5.4	5.4	5.4	5.4	5.4	5.4	5.3	5.3
5.3	5.3	5.3	5.3	5.3	5.3	5.3	5.2	5.2	5.2
5.2	5.2	5.1	5.1	5.1	5.1	5.0	5.0	5.0	5.0
5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.8	4.8
4.8	4.8	4.7	4.7	4.7	4.7	4.6	4.6	4.6	4.6
4.5	4.5	4.5	4.5	4.4	4.4	4.4	4.4	4.4	4.4
4.4	4.3	4.3	4.3	4.3	4.3	4.3	4.3	4.3	4.3
4.2	4.2	4.2	4.2	4.1	4.1	4.1	4.1	4.0	4.0
4.0	4.0	4.0	4.0	4.0	4.0	3.9	3.9	3.9	3.9
3.8	3.8	3.8	3.8	3.8	3.7	3.7	3.7	3.7	3.7
3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.5	3.5	3.5
3.5	3.5	3.5	3.5	3.5	3.5	3.4	3.4	3.4	3.4
3.4	3.4	3.4	3.3	3.3	3.3	3.3	3.3	3.3	3.3
3.3	3.3	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.1
3.1	3.1	3.1	3.1	3.1	3.1	3.1	3.0	3.0	3.0
3.0	3.0	3.0	3.0	3.0	2.9	2.9	2.9	2.9	2.9
2.9	2.9	2.8	2.8	2.8	2.8	2.8	2.8	2.8	2.8
2.8	2.7	2.7	2.7	2.7	2.7	2.7	2.7	2.7	2.6
2.6	2.6	2.6	2.6	2.6	2.6	2.6	2.6	2.6	2.6
2.6	2.6	2.6	2.6	2.6	2.5	2.5	2.5	2.5	2.5
2.5	2.5	2.5	2.4	2.4	2.4	2.4	2.4	2.4	2.4
2.3	2.3	2.3	2.3	2.3	2.3	2.3	2.3	2.3	2.3

2.3 2.3 2.3 2.3 2.3

*** *** *** *** ***

HYDROGRAPH AT STATION E4

TOTAL RAINFALL = 7.46, TOTAL LOSS = 4.55, TOTAL EXCESS = 2.92

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	259.85-HR	
158.	154.75	103.	45.	20.	8.	
		(INCHES)	0.837	1.456	1.968	2.916
		(AC-FT)	51.	89.	121.	179.

CUMULATIVE AREA = 1.15 SQ MI

301 KK

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ES

302 IN

TIME DATA FOR INPUT TIME SERIES

JXMIN 60 TIME INTERVAL IN MINUTES

JXDATE 31DEC99 STARTING DATE

JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

329 BA

SUBBASIN CHARACTERISTICS

TAREA, 0.84 SUBBASIN AREA

PRECIPITATION DATA

304 PB

STORM 7.46 BASIN TOTAL PRECIPITATION

330 LU

UNIFORM LOSS RATE

STRTL 0.20 INITIAL LOSS

CNSTL 0.07 UNIFORM LOSS RATE

RTIMP 2.00 PERCENT IMPERVIOUS AREA

329 UI

INPUT UNITGRAPH, 472 ORDINATES, VOLUME = 1.00

2.8	5.6	8.4	11.3	15.3	19.4	23.4	27.5	31.6	35.9
40.1	44.3	49.0	53.5	58.2	62.5	65.9	69.4	72.8	77.2
83.6	89.9	96.2	102.0	107.0	112.1	117.1	122.7	129.0	135.2
141.5	147.9	154.4	160.9	167.5	174.4	181.7	188.9	196.1	203.4
210.7	218.0	225.4	233.7	242.4	251.1	259.9	269.1	278.5	287.8
297.2	305.9	314.4	323.0	331.5	334.1	336.1	338.2	340.2	341.4
342.5	343.7	344.7	343.5	342.4	341.3	340.0	338.0	335.9	333.8
330.9	324.1	317.3	310.5	303.7	296.6	289.6	282.6	275.6	268.5
261.4	254.3	247.5	241.3	235.1	228.9	223.7	219.8	215.9	212.0
208.1	204.3	200.4	196.6	192.3	187.7	183.2	178.6	175.2	172.4
169.6	166.8	163.9	160.9	158.0	155.1	152.1	149.1	146.0	143.0
140.6	138.3	136.1	133.8	131.5	129.1	126.8	124.5	122.4	120.4
118.3	116.3	114.2	112.1	109.9	107.9	106.5	105.0	103.6	102.1
100.4	98.7	97.0	95.3	94.0	92.6	91.3	89.9	88.5	87.1
85.7	84.3	83.1	81.8	80.5	79.3	78.3	77.2	76.2	75.2
74.3	73.4	72.4	71.4	70.3	69.2	68.1	67.2	66.3	65.4
64.6	63.7	63.0	62.2	61.4	60.6	59.8	59.0	58.2	57.6
57.0	56.5	55.9	55.1	54.2	53.3	52.4	51.9	51.5	51.0
50.5	49.7	48.9	48.1	47.4	47.0	46.7	46.3	45.9	45.3
44.7	44.1	43.5	43.3	43.0	42.8	42.4	42.0	41.6	41.1
40.7	40.4	40.1	39.8	39.5	39.1	38.7	38.3	37.9	37.5
37.1	36.7	36.3	36.0	35.6	35.3	35.0	34.7	34.5	34.2
33.9	33.5	33.2	32.8	32.5	32.2	31.9	31.6	31.4	31.1
30.9	30.7	30.4	30.1	29.8	29.5	29.2	28.9	28.6	28.3
28.1	27.9	27.7	27.5	27.2	26.9	26.6	26.3	26.2	26.0
25.8	25.6	25.3	25.0	24.7	24.4	24.3	24.2	24.0	23.9
23.5	23.2	22.9	22.7	22.5	22.3	22.1	22.0	21.7	21.5
21.3	21.1	21.1	21.0	20.9	20.7	20.4	20.2	19.9	19.7
19.5	19.4	19.2	19.0	18.8	18.6	18.5	18.3	18.1	18.0
17.8	17.6	17.5	17.4	17.2	17.1	17.0	16.9	16.7	16.5
16.4	16.2	16.0	15.9	15.8	15.6	15.5	15.4	15.2	15.1
15.0	14.8	14.7	14.5	14.4	14.3	14.2	14.1	13.9	13.8
13.6	13.5	13.4	13.3	13.1	13.0	12.9	12.8	12.7	12.6
12.5	12.3	12.1	12.0	11.8	11.8	11.7	11.6	11.5	11.3
11.2	11.1	11.0	10.9	10.8	10.7	10.6	10.5	10.4	10.4
10.3	10.2	10.1	10.0	9.9	9.8	9.7	9.6	9.6	9.5
9.4	9.3	9.2	9.1	9.0	8.9	8.9	8.8	8.7	8.6
8.6	8.5	8.5	8.4	8.4	8.4	8.3	8.2	8.2	8.1
8.0	7.9	7.9	7.8	7.8	7.7	7.6	7.5	7.4	7.4
7.3	7.2	7.1	7.0	7.0	6.9	6.9	6.9	6.8	6.8
6.7	6.7	6.6	6.5	6.4	6.3	6.3	6.3	6.2	6.1

6.0	5.9	5.9	5.8	5.8	5.7	5.7	5.6	5.6	5.6
5.5	5.5	5.4	5.4	5.3	5.3	5.3	5.2	5.2	5.1
5.1	5.0	5.0	4.9	4.9	4.9	4.8	4.8	4.7	4.7
4.6	4.6	4.6	4.5	4.5	4.4	4.4	4.3	4.3	4.2
4.2	4.2	4.1	4.1	4.1	4.1	4.1	4.0	4.0	4.0
3.9	3.9	3.8	3.8	3.8	3.7	3.7	3.6	3.6	3.6
3.6	3.6								

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HYDROGRAPH AT STATION E5

TOTAL RAINFALL = 7.46, TOTAL LOSS = 4.55, TOTAL EXCESS = 2.92

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	259.85-HR	
161.	153.62	85.	34.	15.	6.	
		(INCHES)	0.945	1.484	1.969	2.916
		(AC-FT)	42.	67.	88.	131.

CUMULATIVE AREA = 0.84 SQ MI

380 KK * I5CROS *

382 HC HYDROGRAPH COMBINATION
ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION I5CROS

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	259.85-HR	
415.	153.85	255.	106.	47.	19.	
		(INCHES)	0.883	1.466	1.968	2.916
		(AC-FT)	127.	210.	282.	418.

CUMULATIVE AREA = 2.69 SQ MI

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	E2	52.	153.58	28.	11.	5.	0.27		
HYDROGRAPH AT	E1	127.	153.78	70.	28.	12.	0.70		
HYDROGRAPH AT	E4	158.	154.75	103.	45.	20.	1.15		
HYDROGRAPH AT	E5	161.	153.62	85.	34.	15.	0.84		
3 COMBINED AT	I5CROS	415.	153.85	255.	106.	47.	2.69		

*** NORMAL END OF HEC-1L ***

**7. APPENDIX B –
Ultimate Conditions SacCalc Model**


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INPUT FILENAME ==>J:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\SACCALC\Developed\sc.dat
OUTPUT FILENAME ==>J:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\SACCALC\Developed\sc.hecout
DSS FILENAME ==>hcalc.dss
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1L) *
* JULY 1998 *
* VERSION 4.1(L) *
* RUN DATE 12JUL05 TIME 19:20:27 *
*
*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
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Special version of HEC-1 with Extra-large array
 Modified by David Ford Consulting Engineers (2000)
 Program dimensions:
 Number of hydrograph ordinates: 20000
 Unit hydrograph ordinates: 3000

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIME- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

* SacCalc, developed by David Ford Consulting Engineers
 * File generated 07/12/2005 19:20:26
 *

1 ID 10010 & 1010 s1002441024
 * Lag computation for station A
 * Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
 * Basin "n" composition (based on Table 7-1)

Developed				Undeveloped			
n	adj	n	fract	n	adj	n	fract
0.030	0.0300	0.0000	0.067	0.0670	0.0000		
0.031	0.0310	0.3757	0.070	0.0700	0.0000		
0.032	0.0320	0.0000	0.071	0.0710	0.0000		
0.033	0.0330	0.0000	0.072	0.0720	0.0000		
0.034	0.0340	0.0000	0.073	0.0730	0.0000		
0.035	0.0350	0.1751	0.074	0.0740	0.0000		
0.037	0.0370	0.0000	0.076	0.0760	0.0000		
0.040	0.0400	0.1337	0.080	0.0800	0.1350		
0.042	0.0420	0.0000	0.084	0.0840	0.0000		
0.046	0.0460	0.0000	0.088	0.0880	0.0000		
0.050	0.0500	0.0000	0.090	0.0900	0.0000		
0.053	0.0530	0.0000	0.093	0.0930	0.0000		
0.056	0.0560	0.0000	0.096	0.0960	0.0000		
0.060	0.0600	0.0000	0.100	0.1000	0.0000		
0.065	0.0650	0.0414	0.110	0.1100	0.0535		
0.070	0.0700	0.0856	0.115	0.1150	0.0000		
0.075	0.0750	0.0000	0.120	0.1200	0.0000		
0.080	0.0800	0.0000	0.150	0.1500	0.0000		

* Equation (7-1) with L= 0.6705, Lc= 0.2765, S= 13.200, n=0.0485
 * Resulting lag: 28.3 minutes
 * Lag computation for station B
 * Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
 * Basin "n" composition (based on Table 7-1)

Developed				Undeveloped			
n	adj	n	fract	n	adj	n	fract
0.030	0.0300	0.0000	0.067	0.0670	0.0000		
0.031	0.0310	0.0000	0.070	0.0700	0.0000		
0.032	0.0320	0.0000	0.071	0.0710	0.0000		
0.033	0.0330	0.0000	0.072	0.0720	0.0000		
0.034	0.0340	0.0000	0.073	0.0730	0.0000		

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*      0.035 0.0350 0.7942      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.0289      0.080 0.0800 0.0000
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.1480      0.110 0.1100 0.0000
*      0.070 0.0700 0.0144      0.115 0.1150 0.0144
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 3.3504, Lc= 0.1186, S= 13.200, n=0.0412
* Resulting lag: 14.7 minutes
* Lag computation for station C
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed                               Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.0000      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.0000      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 0.6675      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.0914      0.080 0.0800 0.1015
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.1041      0.110 0.1100 0.0000
*      0.070 0.0700 0.0178      0.115 0.1150 0.0178
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.4485, Lc= 0.1420, S= 13.200, n=0.0452
* Resulting lag: 18.6 minutes
* Lag computation for station D
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed                               Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.0000      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.0000      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 0.3054      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.4966      0.080 0.0800 0.0000
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.1443      0.110 0.1100 0.0000
*      0.070 0.0700 0.0201      0.115 0.1150 0.0336
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.4555, Lc= 0.1290, S= 13.200, n=0.0452
* Resulting lag: 18.1 minutes
* Lag computation for station E
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed                               Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.2282      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.0000      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 0.3983      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.2075      0.080 0.0800 0.0000
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.1660      0.110 0.1100 0.0000
*      0.070 0.0700 0.0000      0.115 0.1150 0.0000
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----

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* Equation (7-1) with L= 2.3797, Lc= 0.1150, S= 10.560, n=0.0399
* Resulting lag: 15.0 minutes
* Lag computation for station F
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
* Developed Undeveloped
* n adj n fract n adj n fract
*-----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.0000 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000
* 0.035 0.0350 0.0000 0.074 0.0740 0.0000
* 0.037 0.0370 0.0000 0.076 0.0760 0.0000
* 0.040 0.0400 0.9898 0.080 0.0800 0.0000
* 0.042 0.0420 0.0000 0.084 0.0840 0.0000
* 0.046 0.0460 0.0000 0.088 0.0880 0.0000
* 0.050 0.0500 0.0000 0.090 0.0900 0.0000
* 0.053 0.0530 0.0000 0.093 0.0930 0.0000
* 0.056 0.0560 0.0000 0.096 0.0960 0.0000
* 0.060 0.0600 0.0000 0.100 0.1000 0.0000
* 0.065 0.0650 0.0000 0.110 0.1100 0.0000
* 0.070 0.0700 0.0051 0.115 0.1150 0.0051
* 0.075 0.0750 0.0000 0.120 0.1200 0.0000
* 0.080 0.0800 0.0000 0.150 0.1500 0.0000
*-----
* Equation (7-1) with L= 0.3589, Lc= 0.1383, S= 10.560, n=0.0405
* Resulting lag: 15.9 minutes
* Lag computation for station G
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
* Developed Undeveloped
* n adj n fract n adj n fract
*-----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.0000 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000
* 0.035 0.0350 0.0000 0.074 0.0740 0.0000
* 0.037 0.0370 0.0000 0.076 0.0760 0.0000
* 0.040 0.0400 0.7583 0.080 0.0800 0.0000
* 0.042 0.0420 0.0000 0.084 0.0840 0.0000
* 0.046 0.0460 0.0000 0.088 0.0880 0.0000
* 0.050 0.0500 0.0000 0.090 0.0900 0.0000
* 0.053 0.0530 0.0000 0.093 0.0930 0.0000
* 0.056 0.0560 0.0000 0.096 0.0960 0.0000
* 0.060 0.0600 0.0000 0.100 0.1000 0.0000
* 0.065 0.0650 0.1730 0.110 0.1100 0.0000
* 0.070 0.0700 0.0433 0.115 0.1150 0.0254
* 0.075 0.0750 0.0000 0.120 0.1200 0.0000
* 0.080 0.0800 0.0000 0.150 0.1500 0.0000
*-----
* Equation (7-1) with L= 0.3930, Lc= 0.1231, S= 13.200, n=0.0475
* Resulting lag: 17.8 minutes
* Lag computation for station H
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
* Developed Undeveloped
* n adj n fract n adj n fract
*-----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.0000 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000
* 0.035 0.0350 0.1213 0.074 0.0740 0.0000
* 0.037 0.0370 0.0000 0.076 0.0760 0.0000
* 0.040 0.0400 0.6798 0.080 0.0800 0.0845
* 0.042 0.0420 0.0000 0.084 0.0840 0.0000
* 0.046 0.0460 0.0000 0.088 0.0880 0.0000
* 0.050 0.0500 0.0000 0.090 0.0900 0.0000
* 0.053 0.0530 0.0000 0.093 0.0930 0.0000
* 0.056 0.0560 0.0000 0.096 0.0960 0.0000
* 0.060 0.0600 0.0000 0.100 0.1000 0.0000
* 0.065 0.0650 0.0000 0.110 0.1100 0.0000
* 0.070 0.0700 0.0578 0.115 0.1150 0.0564
* 0.075 0.0750 0.0000 0.120 0.1200 0.0000
* 0.080 0.0800 0.0000 0.150 0.1500 0.0000
*-----
* Equation (7-1) with L= 0.6178, Lc= 0.2131, S= 10.560, n=0.0487
* Resulting lag: 26.4 minutes
* Lag computation for station I
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
* Developed Undeveloped
* n adj n fract n adj n fract
*-----
* 0.030 0.0300 0.0000 0.067 0.0670 0.0000
* 0.031 0.0310 0.0000 0.070 0.0700 0.0000
* 0.032 0.0320 0.0000 0.071 0.0710 0.0000
* 0.033 0.0330 0.1959 0.072 0.0720 0.0000
* 0.034 0.0340 0.0000 0.073 0.0730 0.0000
* 0.035 0.0350 0.0591 0.074 0.0740 0.0000

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*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.5083      0.080 0.0800 0.0000
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.2089      0.110 0.1100 0.0000
*      0.070 0.0700 0.0092      0.115 0.1150 0.0185
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.6780, Lc= 0.3822, S= 10.560, n=0.0452
* Resulting lag: 30.6 minutes
* Lag computation for station J
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed      Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.1145      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.6964      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 0.0000      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.1164      0.080 0.0800 0.0545
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.0000      0.110 0.1100 0.0000
*      0.070 0.0700 0.0091      0.115 0.1150 0.0091
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.5758, Lc= 0.2036, S= 10.560, n=0.0371
* Resulting lag: 19.3 minutes
* Lag computation for station K
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed      Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.0000      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.0000      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 0.6104      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.0248      0.080 0.0800 0.0521
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.2481      0.110 0.1100 0.0000
*      0.070 0.0700 0.0397      0.115 0.1150 0.0248
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.4909, Lc= 0.2286, S= 10.560, n=0.0483
* Resulting lag: 24.8 minutes
* Lag computation for station L
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*      Developed      Undeveloped
*      n  adj n  fract      n  adj n  fract
* -----
*      0.030 0.0300 0.0000      0.067 0.0670 0.0000
*      0.031 0.0310 0.0000      0.070 0.0700 0.0000
*      0.032 0.0320 0.0000      0.071 0.0710 0.0000
*      0.033 0.0330 0.0000      0.072 0.0720 0.0000
*      0.034 0.0340 0.0000      0.073 0.0730 0.0000
*      0.035 0.0350 1.0000      0.074 0.0740 0.0000
*      0.037 0.0370 0.0000      0.076 0.0760 0.0000
*      0.040 0.0400 0.0000      0.080 0.0800 0.0000
*      0.042 0.0420 0.0000      0.084 0.0840 0.0000
*      0.046 0.0460 0.0000      0.088 0.0880 0.0000
*      0.050 0.0500 0.0000      0.090 0.0900 0.0000
*      0.053 0.0530 0.0000      0.093 0.0930 0.0000
*      0.056 0.0560 0.0000      0.096 0.0960 0.0000
*      0.060 0.0600 0.0000      0.100 0.1000 0.0000
*      0.065 0.0650 0.0000      0.110 0.1100 0.0000
*      0.070 0.0700 0.0000      0.115 0.1150 0.0000
*      0.075 0.0750 0.0000      0.120 0.1200 0.0000
*      0.080 0.0800 0.0000      0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.2784, Lc= 0.0848, S= 10.560, n=0.0350

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* Resulting lag: 10.8 minutes
* Lag computation for station M
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
*      Developed                      Undeveloped
*      n  adj n  fract                n  adj n  fract
* -----
*      0.030 0.0300 1.0000            0.067 0.0670 0.0000
*      0.031 0.0310 0.0000            0.070 0.0700 0.0000
*      0.032 0.0320 0.0000            0.071 0.0710 0.0000
*      0.033 0.0330 0.0000            0.072 0.0720 0.0000
*      0.034 0.0340 0.0000            0.073 0.0730 0.0000
*      0.035 0.0350 0.0000            0.074 0.0740 0.0000
*      0.037 0.0370 0.0000            0.076 0.0760 0.0000
*      0.040 0.0400 0.0000            0.080 0.0800 0.0000
*      0.042 0.0420 0.0000            0.084 0.0840 0.0000
*      0.046 0.0460 0.0000            0.088 0.0880 0.0000
*      0.050 0.0500 0.0000            0.090 0.0900 0.0000
*      0.053 0.0530 0.0000            0.093 0.0930 0.0000
*      0.056 0.0560 0.0000            0.096 0.0960 0.0000
*      0.060 0.0600 0.0000            0.100 0.1000 0.0000
*      0.065 0.0650 0.0000            0.110 0.1100 0.0000
*      0.070 0.0700 0.0000            0.115 0.1150 0.0000
*      0.075 0.0750 0.0000            0.120 0.1200 0.0000
*      0.080 0.0800 0.0000            0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.8854, Lc= 0.4290, S= 4.752, n=0.0300
* Resulting lag: 26.3 minutes
* Lag computation for station N
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
*      Developed                      Undeveloped
*      n  adj n  fract                n  adj n  fract
* -----
*      0.030 0.0300 0.0000            0.067 0.0670 0.0000
*      0.031 0.0310 0.0000            0.070 0.0700 0.0000
*      0.032 0.0320 0.0000            0.071 0.0710 0.0000
*      0.033 0.0330 0.0000            0.072 0.0720 0.0000
*      0.034 0.0340 0.0000            0.073 0.0730 0.0000
*      0.035 0.0350 0.2500            0.074 0.0740 0.0000
*      0.037 0.0370 0.0000            0.076 0.0760 0.0000
*      0.040 0.0400 0.4917            0.080 0.0800 0.0000
*      0.042 0.0420 0.0000            0.084 0.0840 0.0000
*      0.046 0.0460 0.0000            0.088 0.0880 0.0000
*      0.050 0.0500 0.0000            0.090 0.0900 0.0000
*      0.053 0.0530 0.0000            0.093 0.0930 0.0000
*      0.056 0.0560 0.0000            0.096 0.0960 0.0000
*      0.060 0.0600 0.0000            0.100 0.1000 0.0000
*      0.065 0.0650 0.1708            0.110 0.1100 0.0000
*      0.070 0.0700 0.0875            0.115 0.1150 0.0000
*      0.075 0.0750 0.0000            0.120 0.1200 0.0000
*      0.080 0.0800 0.0000            0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.4350, Lc= 0.1388, S= 10.560, n=0.0456
* Resulting lag: 19.1 minutes
* Lag computation for station O
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
*      Developed                      Undeveloped
*      n  adj n  fract                n  adj n  fract
* -----
*      0.030 0.0300 0.0000            0.067 0.0670 0.0000
*      0.031 0.0310 0.0000            0.070 0.0700 0.0000
*      0.032 0.0320 0.0000            0.071 0.0710 0.0000
*      0.033 0.0330 0.0000            0.072 0.0720 0.0000
*      0.034 0.0340 0.0000            0.073 0.0730 0.0000
*      0.035 0.0350 0.0000            0.074 0.0740 0.0000
*      0.037 0.0370 0.0000            0.076 0.0760 0.0000
*      0.040 0.0400 1.0000            0.080 0.0800 0.0000
*      0.042 0.0420 0.0000            0.084 0.0840 0.0000
*      0.046 0.0460 0.0000            0.088 0.0880 0.0000
*      0.050 0.0500 0.0000            0.090 0.0900 0.0000
*      0.053 0.0530 0.0000            0.093 0.0930 0.0000
*      0.056 0.0560 0.0000            0.096 0.0960 0.0000
*      0.060 0.0600 0.0000            0.100 0.1000 0.0000
*      0.065 0.0650 0.0000            0.110 0.1100 0.0000
*      0.070 0.0700 0.0000            0.115 0.1150 0.0000
*      0.075 0.0750 0.0000            0.120 0.1200 0.0000
*      0.080 0.0800 0.0000            0.150 0.1500 0.0000
* -----
* Equation (7-1) with L= 0.2949, Lc= 0.1515, S= 10.560, n=0.0400
* Resulting lag: 15.2 minutes
* Lag computation for station P
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
*      Developed                      Undeveloped
*      n  adj n  fract                n  adj n  fract
* -----
*      0.030 0.0300 0.0000            0.067 0.0670 0.0000
*      0.031 0.0310 0.0000            0.070 0.0700 0.0000
*      0.032 0.0320 0.0000            0.071 0.0710 0.0000
*      0.033 0.0330 0.0000            0.072 0.0720 0.0000
*      0.034 0.0340 0.0000            0.073 0.0730 0.0000
*      0.035 0.0350 0.7945            0.074 0.0740 0.0000
*      0.037 0.0370 0.0000            0.076 0.0760 0.0000

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```

*   0.040 0.0400 0.0000    0.080 0.0800 0.0000
*   0.042 0.0420 0.0000    0.084 0.0840 0.0000
*   0.046 0.0460 0.0000    0.088 0.0880 0.0000
*   0.050 0.0500 0.0000    0.090 0.0900 0.0000
*   0.053 0.0530 0.0000    0.093 0.0930 0.0000
*   0.056 0.0560 0.0000    0.096 0.0960 0.0000
*   0.060 0.0600 0.0000    0.100 0.1000 0.0000
*   0.065 0.0650 0.1187    0.110 0.1100 0.0000
*   0.070 0.0700 0.0868    0.115 0.1150 0.0000
*   0.075 0.0750 0.0000    0.120 0.1200 0.0000
*   0.080 0.0800 0.0000    0.150 0.1500 0.0000
*-----*
* Equation (7-1) with L= 0.3576, Lc= 0.1437, S= 10.560, n=0.0416
* Resulting lag: 16.5 minutes
* Lag computation for station OFF-N
* Lag frequency factor of 1.0 (Table 7-6) for frequency 10 years will be appl
* Basin "n" composition (based on Table 7-1)
*
*   Developed                               Undeveloped
*   n   adj n   fract                       n   adj n   fract
*-----*-----*-----*-----*-----*-----*-----*-----*
*   0.030 0.0300 0.0000    0.067 0.0670 0.0000
*   0.031 0.0310 0.0000    0.070 0.0700 0.0000
*   0.032 0.0320 0.0000    0.071 0.0710 0.0000
*   0.033 0.0330 0.0000    0.072 0.0720 0.0000
*   0.034 0.0340 0.0000    0.073 0.0730 0.0000
*   0.035 0.0350 0.0000    0.074 0.0740 0.0000
*   0.037 0.0370 0.0000    0.076 0.0760 0.0000
*   0.040 0.0400 0.0000    0.080 0.0800 0.0000
*   0.042 0.0420 0.0000    0.084 0.0840 0.0000
*   0.046 0.0460 0.0000    0.088 0.0880 0.0000
*   0.050 0.0500 0.0000    0.090 0.0900 0.0000
*   0.053 0.0530 0.0000    0.093 0.0930 0.0000
*   0.056 0.0560 0.0000    0.096 0.0960 0.0000
*   0.060 0.0600 0.0000    0.100 0.1000 0.0000
*   0.065 0.0650 0.0000    0.110 0.1100 0.0000
*   0.070 0.0700 0.5000    0.115 0.1150 0.5000
*   0.075 0.0750 0.0000    0.120 0.1200 0.0000
*   0.080 0.0800 0.0000    0.150 0.1500 0.0000
*-----*
* Equation (7-1) with L= 0.9470, Lc= 0.3788, S= 5.280, n=0.0925
* Resulting lag: 78.2 minutes
* End of lag computations
*
*   NMIN  JXDATE  JXTIME      NQ
*                   HEC-1L INPUT

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1

PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
2     IT      1 31DEC99   2400 14947
*     IPRT    IPLT
3     IO      3      0
*
4     RK      A
*     JXMIN   Time interval for input data
5     IN      60
6     KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.913
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min      0.2500  0.2500  0.2500
* 10 min     0.3600  0.3600  0.3600
* 15 min     0.4300  0.4300  0.4300
* 30 min     0.5700  0.5700  0.5700
* 1 hour     0.7700  0.7700  0.7700
* 2 hours    1.0400  1.0400  1.0315
* 3 hours    1.2300  1.2300  1.2200
* 6 hours    1.6500  1.6500  1.6366
* 12 hours   2.2500  2.2500  2.2500
* 24 hours   2.9800  2.9800  2.9800
* 36 hours   3.5400  3.5400  3.5046
* 2 days     3.9500  3.9500  3.9500
* 3 days     4.6500  4.6500  4.6500
* 5 days     5.7600  5.7600  5.7024
* 10 days    7.5400  7.5400  7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
7     PR      0
8     PI      0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
9     PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
10    PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
11    PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
12    PI      0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
13    PI      0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
14    PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
15    PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
16    PI      0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
17    PI      0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
18    PI      0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224

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19      PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
20      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
21      PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
22      PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
23      PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
24      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
25      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
26      PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493

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HEC-1L INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
27      PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
28      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
29      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
30      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
31      PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
32      BA 0.1169
* STRL CNSTL RTIMP
33      LU 0.20 0.061 60.151
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 28.3 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 28.83720
* Volume of runoff (Step 4) V= 3.142639
34      UI 2.90 6.51 10.71 15.06 19.69 24.18 27.76 33.67 39.74 44.96
35      UI 51.33 57.94 64.71 72.19 79.73 87.50 96.53 106.03 115.51 124.36
36      UI 127.74 129.51 130.69 129.68 128.03 125.67 118.54 111.48 104.21 96.86
37      UI 90.17 84.40 80.36 76.37 72.09 67.37 64.39 61.41 58.37 55.24
38      UI 52.59 50.23 47.82 45.62 43.48 41.30 39.71 38.10 36.33 34.92
39      UI 33.49 32.05 30.73 29.57 28.51 27.55 26.46 25.40 24.49 23.67
40      UI 22.85 22.04 21.45 20.68 19.82 19.32 18.61 17.90 17.53 16.96
41      UI 16.45 16.18 15.74 15.35 15.03 14.62 14.21 13.81 13.45 13.15
42      UI 12.85 12.49 12.17 11.88 11.65 11.34 11.03 10.72 10.53 10.27
43      UI 9.97 9.79 9.51 9.25 9.11 8.82 8.56 8.38 8.15 8.00
44      UI 7.89 7.62 7.41 7.22 7.04 6.86 6.69 6.56 6.42 6.27
45      UI 6.09 5.95 5.81 5.68 5.54 5.40 5.27 5.13 5.00 4.87
46      UI 4.78 4.61 4.47 4.38 4.24 4.13 4.04 3.95 3.86 3.77
47      UI 3.68 3.59 3.50 3.41 3.32 3.25 3.20 3.16 3.09 3.00
48      UI 2.96 2.88 2.79 2.70 2.64 2.60 2.55 2.47 2.40 2.35
49      UI 2.26 2.20 2.15 2.11 2.06 2.01 1.97 1.92 1.88 1.83
50      UI 1.79 1.74 1.70 1.65 1.61 1.57 1.57 1.54 1.49 1.44
51      UI 1.40 1.37 1.37
52      ZW C=FLOW P=010YR-1DDY A=GREENBRI
*

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53      KK B
* JXMIN Time interval for input data
54      IN 60
55      KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*

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```

* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
56      PB 0
57      PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
58      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
59      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
60      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
61      PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
62      PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
63      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
64      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

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65      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
66      PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
67      PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
68      PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
69      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
70      PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
71      PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
72      PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
73      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
74      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
75      PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523 0.0672 0.0970 0.1493
76      PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
77      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
78      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
79      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
80      PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
81      BA 0.0433
* STRTL CNSTL RTIMP
82      LU 0.20 0.062 57.838
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 14.7 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 15.21310
* Volume of runoff (Step 4) Vr = 1.163785
83      UI 4.266 9.933 16.116 21.721 29.648 37.728 46.837 56.841 68.140 80.809
84      UI 89.443 91.531 90.410 85.565 76.047 66.329 58.610 53.270 47.241 43.300
85      UI 39.201 35.757 32.628 29.766 27.467 25.216 23.323 21.504 20.029 18.681
86      UI 17.346 16.237 15.239 14.230 13.438 12.502 11.842 11.349 10.791 10.315
87      UI 9.772 9.304 8.893 8.449 8.119 7.696 7.383 7.004 6.705 6.417
88      UI 6.050 5.792 5.583 5.281 5.039 4.797 4.599 4.406 4.191 4.010
89      UI 3.829 3.648 3.467 3.325 3.125 2.970 2.838 2.718 2.597 2.476
90      UI 2.355 2.265 2.205 2.098 2.010 1.890 1.825 1.740 1.659 1.553
91      UI 1.492 1.432 1.372 1.311 1.251 1.191 1.130 1.102 1.055 0.995
92      UI 0.964
93      ZW C=FLOW P=010YR-10DY A=GREENBERI
*

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HEC-1L INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

94      KK C
* JXMIN Time interval for input data
95      IN 60
96      KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration-----Regional-----Elev-----Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
97      PB 0
98      PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
99      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
100     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
101     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
102     PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
103     PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
104     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
105     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
106     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
107     PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
108     PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
109     PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
110     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
111     PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
112     PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
113     PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
114     PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

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115 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 116 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
 117 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
 118 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 119 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 120 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075

HEC-1L INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
 *
 * Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TAREA subbasin area (sq mi)
 122 BA 0.0616
 * STRTL CNSTL RTIMP
 123 LU 0.20 0.063 56.962
 *
 * Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 18.6 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 19.06322
 * Volume of runoff (Step 4) V= 1.655347
 124 UI 3.57 8.62 13.94 19.41 24.56 31.99 38.77 46.71 55.23 64.30
 125 UI 74.33 85.66 96.73 102.08 103.85 103.14 100.80 93.26 84.54 75.69
 126 UI 68.00 63.14 58.22 52.91 49.38 45.68 42.20 39.34 36.58 33.98
 127 UI 31.77 29.78 27.91 26.18 24.55 23.16 21.98 20.65 19.52 18.53
 128 UI 17.55 16.81 15.77 15.11 14.24 13.72 13.08 12.68 12.20 11.77
 129 UI 11.28 10.81 10.44 10.04 9.64 9.34 8.98 8.60 8.36 7.99
 130 UI 7.76 7.39 7.21 6.85 6.62 6.38 6.21 5.92 5.70 5.48
 131 UI 5.29 5.13 4.93 4.75 4.58 4.42 4.25 4.09 3.93 3.81
 132 UI 3.60 3.49 3.33 3.22 3.11 3.00 2.89 2.78 2.67 2.58
 133 UI 2.53 2.45 2.37 2.28 2.17 2.10 2.04 1.95 1.88 1.78
 134 UI 1.72 1.67 1.61 1.56 1.50 1.45 1.39 1.34 1.28 1.25
 135 UI 1.23 1.17 1.12 1.09
 136 ZW C=FLOW P=010YR-10DY A=GREENBRI
 *

137 KK D
 * JXMIN Time interval for input data
 138 IN 60
 139 KM
 *
 * Design storm construction details
 *
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 1.813
 * multiplier from table 4-4: 1.0000
 *

* Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration---Regional-----Elev-----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2500
 * 10 min 0.3600 0.3600 0.3600
 * 15 min 0.4300 0.4300 0.4300
 * 30 min 0.5700 0.5700 0.5700
 * 1 hour 0.7700 0.7700 0.7700
 * 2 hours 1.0400 1.0400 1.0315
 * 3 hours 1.2300 1.2300 1.2200
 * 6 hours 1.6500 1.6500 1.6366
 * 12 hours 2.2500 2.2500 2.2500
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646

HEC-1L INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

140 PB 0
 141 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
 142 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 143 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 144 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149
 145 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
 146 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 147 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 148 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 149 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 150 PI 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448
 151 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
 152 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000
 153 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
 154 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
 155 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
 156 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
 157 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 158 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 159 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
 160 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000

```

161      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
162      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
163      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
164      PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
165      BA 0.0466
* STRTL CNSTL RTIMP
166      LU 0.20 0.067 47.037
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 18.1 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 18.57796
* Volume of runoff (Step 4) V= 1.252014
167      UI 2.873 6.902 11.165 15.414 19.858 25.588 31.201 37.574 44.506 51.751
168      UI 60.110 69.329 77.880 79.895 80.715 79.393 76.031 69.207 62.206 55.583
169      UI 50.704 46.857 42.534 39.361 36.464 33.468 31.195 28.922 26.867 24.958
170      UI 23.431 21.878 20.508 19.188 18.063 17.110 16.061 15.156 14.365 13.594
171      UI 12.989 12.197 11.635 11.001 10.545 10.104 9.746 9.392 9.027 8.635
172      UI 8.280 8.011 7.667 7.373 7.138 6.833 6.573 6.345 6.103 5.857
173      UI 5.661 5.409 5.204 4.994 4.895 4.642 4.468 4.293 4.131 4.000
174      UI 3.852 3.698 3.568 3.437 3.307 3.176 3.046 2.954 2.786 2.694
175      UI 2.572 2.485 2.398 2.311 2.223 2.136 2.049 1.992 1.948 1.869
176      UI 1.821 1.735 1.653 1.610 1.555 1.482 1.421 1.355 1.311 1.267
177      UI 1.224 1.180 1.137 1.093 1.050 1.006 0.970 0.960 0.916 0.873
178      UI 0.849
179      ZW C=FLOW F=010YR-10DY A=GREENBRI
*

```

1

HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

180      KK 8
* JXMIN Time interval for input data
181      IN 60
182      KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration----Regional----Elev----Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
183      PB 0
184      PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
185      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
186      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
187      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0149 0.0224
188      PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
189      PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
190      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
191      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
192      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
193      PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
194      PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
195      PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
196      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
197      PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
198      PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
199      PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
200      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
201      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
202      PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
203      PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
204      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
205      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
206      PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
*

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1

HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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207 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
208 BA 0.0377
* STRTL CNSTL RTIMP
209 LU 0.20 0.059 60.768
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 15.0 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 15.50161
* Volume of runoff (Step 4) V= 1.012535
210 UI 3.555 8.297 13.458 17.994 24.763 31.391 38.884 47.252 56.463 67.056
211 UI 75.874 77.854 77.595 75.140 67.234 59.093 51.633 47.003 42.232 38.233
212 UI 34.861 31.628 28.972 26.519 24.243 22.447 20.736 19.140 17.755 16.624
213 UI 15.406 14.416 13.506 12.766 11.847 11.168 10.546 9.909 9.556 9.122
214 UI 8.697 8.250 7.883 7.519 7.160 6.880 6.526 6.282 5.962 5.700
215 UI 5.471 5.159 4.942 4.768 4.516 4.311 4.109 3.939 3.783 3.596
216 UI 3.445 3.293 3.141 2.990 2.872 2.692 2.574 2.451 2.350 2.249
217 UI 2.147 2.046 1.952 1.902 1.821 1.759 1.658 1.582 1.531 1.442
218 UI 1.371 1.301 1.251 1.200 1.150 1.099 1.048 0.998 0.947 0.936
219 UI 0.885 0.835 0.823
220 ZW C=FLOW F=010YR-10DY A=GREENBRI
*

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221 KK DUMMY1
222 KM
223 HC 5
224 ZW C=FLOW
*

```

```

225 KK F
* JXMIN Time interval for input data
226 IN 60
227 KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration-----Regional-----Elev-----Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
REC-1L INPRT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
228 FB 0
229 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
230 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
231 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
232 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149
233 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
234 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
235 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
236 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
237 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
238 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
239 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
240 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
241 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
242 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
243 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
244 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
245 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
246 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
247 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
248 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
249 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
250 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
251 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
252 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)

```

```

* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
253 BA 0.0458
* STRTL CNSTL RTIMP
254 LU 0.20 0.070 49.509
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 15.9 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 16.40914
* Volume of runoff (Step 4) V= 1.231007
255 UI 3.787 8.913 14.446 19.182 26.594 33.298 41.287 50.002 59.141 70.142
256 UI 81.534 88.062 89.842 88.719 84.767 76.150 67.308 59.265 54.244 49.145
257 UI 44.512 40.890 37.147 34.283 31.502 28.893 26.873 24.804 23.088 21.408
258 UI 19.994 18.812 17.490 16.433 15.445 14.655 13.636 12.933 12.186 11.543
259 UI 11.125 10.608 10.187 9.693 9.237 8.888 8.462 8.124 7.784 7.400
260 UI 7.151 6.819 6.518 6.281 5.935 5.704 5.494 5.259 5.012 4.793
261 UI 4.589 4.425 4.225 4.050 3.886 3.721 3.557 3.392 3.268 3.077
262 UI 2.943 2.813 2.703 2.593 2.484 2.374 2.264 2.202 2.135 2.048
263 UI 1.960 1.850 1.793 1.721 1.638 1.547 1.484 1.429 1.374 1.319
264 UI 1.264 1.209 1.154 1.100 1.080 1.035 0.980 0.945
265 ZW C=FLOW F=010YR-10DY A=GREENBRI
*

```

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1

HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

266 KK G
* JXMIN Time interval for input data
267 IN 60
268 KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
269 PB 0
270 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
271 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
272 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
273 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
274 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
275 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
276 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
277 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
278 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
279 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
280 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
281 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
282 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
283 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
284 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
285 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
286 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
287 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
288 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
289 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
290 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
291 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
292 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075

```

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1

HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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293 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
294 BA 0.0614
* STRTL CNSTL RTIMP

```

295 LU 0.20 0.070 38.916
 *
 * Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 17.8 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 18.33008
 * Volume of runoff (Step 4) V= 1.651146
 296 UI 3.91 9.38 15.17 20.85 27.11 34.74 42.49 51.16 60.66 70.49
 297 UI 82.07 94.59 104.62 107.08 107.58 105.53 99.55 90.24 80.72 72.06
 298 UI 66.30 61.10 55.18 51.37 47.39 43.66 40.57 37.62 34.82 32.58
 299 UI 30.37 28.46 26.58 24.90 23.51 22.21 20.84 19.71 18.65 17.75
 300 UI 16.72 15.94 14.94 14.42 13.64 13.25 12.70 12.25 11.72 11.22
 301 UI 10.82 10.38 9.96 9.64 9.24 8.84 8.57 8.21 7.91 7.60
 302 UI 7.30 6.99 6.72 6.56 6.24 6.00 5.76 5.54 5.36 5.16
 303 UI 4.95 4.77 4.60 4.42 4.24 4.07 3.94 3.72 3.59 3.43
 304 UI 3.31 3.19 3.07 2.96 2.84 2.72 2.65 2.59 2.48 2.41
 305 UI 2.29 2.20 2.14 2.05 1.96 1.87 1.79 1.73 1.67 1.62
 306 UI 1.56 1.50 1.44 1.38 1.32 1.30 1.26 1.20 1.14
 307 ZW C=FLOW F=010YR-10DY A=GREENBRI
 *

308 KK H
 * JXMIN Time interval for input data
 309 IN 60
 310 KM
 *

* Design storm construction details
 *
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 1.813
 * multiplier from table 4-4: 1.0000
 *
 * Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration----Regional-----Elev-----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2500
 * 10 min 0.3600 0.3600 0.3600
 * 15 min 0.4300 0.4300 0.4300
 * 30 min 0.5700 0.5700 0.5700
 * 1 hour 0.7700 0.7700 0.7700
 * 2 hours 1.0400 1.0400 1.0315
 * 3 hours 1.2300 1.2300 1.2200
 * 6 hours 1.6500 1.6500 1.6366
 * 12 hours 2.2500 2.2500 2.2500
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646
 HEC-1L INPUT

1

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 311 FB 0
 312 FI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
 313 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 314 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 315 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
 316 FI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
 317 FI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 318 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 319 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 320 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 321 FI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
 322 FI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
 323 FI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
 324 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
 325 FI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
 326 FI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
 327 FI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
 328 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 329 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 330 FI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
 331 FI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
 332 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 333 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 334 FI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
 335 FI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149

* Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TAREA subbasin area (sq mi)

336 BA 0.1108
 * STRTL CNSTL RTIMP
 337 LU 0.20 0.069 46.942
 *

* Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 26.4 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 26.89606
 * Volume of runoff (Step 4) V= 2.978785
 338 UI 3.16 7.23 11.84 16.58 21.76 26.15 30.94 38.05 43.83 50.33

339	UI	57.46	64.83	72.90	81.11	89.69	99.52	109.99	120.08	128.45	130.77
340	UI	132.24	132.28	130.85	128.53	121.97	114.20	106.29	98.28	91.03	85.10
341	UI	80.70	76.35	71.43	67.04	63.88	60.57	57.19	54.07	51.50	48.88
342	UI	46.45	44.12	41.74	40.09	38.27	36.46	34.92	33.35	31.85	30.48
343	UI	29.29	28.24	27.09	25.91	24.92	24.02	23.13	22.28	21.64	20.71
344	UI	19.94	19.35	18.46	17.96	17.43	16.78	16.48	16.04	15.61	15.25
345	UI	14.81	14.36	13.94	13.55	13.25	12.88	12.51	12.16	11.91	11.60
346	UI	11.26	10.91	10.71	10.42	10.11	9.91	9.58	9.35	9.15	8.81
347	UI	8.59	8.37	8.15	8.05	7.78	7.54	7.34	7.14	6.95	6.77
348	UI	6.62	6.48	6.29	6.11	5.96	5.82	5.67	5.52	5.37	5.22
349	UI	5.08	4.95	4.84	4.65	4.52	4.40	4.25	4.15	4.06	3.96
350	UI	3.86	3.76	3.66	3.56	3.46	3.36	3.29	3.24	3.19	3.10
351	UI	3.03	2.97	2.87	2.77	2.70	2.65	2.60	2.51	2.43	2.38
352	UI	2.28	2.22	2.17	2.12	2.07	2.02	1.97	1.92	1.87	1.82
353	UI	1.78	1.73	1.68	1.63	1.59	1.59	1.55	1.50	1.45	1.40

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

354 UI 1.40
 355 ZW C=FLOW F=010YR-10DY A=GREENBRI
 *

356 KK I
 * JXMIN Time interval for input data
 357 IN 60
 358 KM
 *

* Design storm construction details
 *
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 1.813
 * multiplier from table 4-4: 1.0000
 *
 * Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration---Regional---Elev---Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2500
 * 10 min 0.3600 0.3600 0.3600
 * 15 min 0.4300 0.4300 0.4300
 * 30 min 0.5700 0.5700 0.5700
 * 1 hour 0.7700 0.7700 0.7700
 * 2 hours 1.0400 1.0400 1.0315
 * 3 hours 1.2300 1.2300 1.2200
 * 6 hours 1.6500 1.6500 1.6366
 * 12 hours 2.2500 2.2500 2.2500
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646

359	FB	0									
360	PI	0.0224	0.0821	0.2015	0.1120	0.0373	0.0224	0.0075	0.0000	0.0000	0.0000
361	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
362	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
363	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224
364	PI	0.0373	0.0523	0.0572	0.0970	0.2239	0.1418	0.0746	0.0597	0.0448	0.0373
365	PI	0.0299	0.0224	0.0149	0.0075	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
366	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
367	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
368	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
369	PI	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224	0.0299	0.0373	0.0448	0.0523
370	PI	0.0672	0.1120	0.3956	0.1642	0.0746	0.0597	0.0448	0.0373	0.0299	0.0224
371	PI	0.0224	0.0149	0.0149	0.0075	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
372	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
373	PI	0.0149	0.0149	0.0149	0.0224	0.0373	0.0448	0.0523	0.0672	0.0746	0.0821
374	PI	0.0970	0.1045	0.1194	0.1269	0.1344	0.1418	0.1568	0.1120	0.0896	0.0672
375	PI	0.2314	0.2911	0.5001	0.2463	0.0373	0.0224	0.0149	0.0075	0.0075	0.0075
376	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
377	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
378	PI	0.0075	0.0149	0.0224	0.0299	0.0373	0.0523	0.0672	0.0970	0.2911	0.1493
379	PI	0.0746	0.0597	0.0523	0.0448	0.0373	0.0299	0.0224	0.0149	0.0075	0.0000

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

380	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
381	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
382	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075
383	PI	0.0149	0.0373	0.0523	0.0746	0.2165	0.1194	0.0597	0.0448	0.0299	0.0149

*
 * Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TARBA subbasin area (sq mi)
 384 BA 0.0845
 * STRTL CNSTL RTIMP
 385 LU 0.20 0.066 46.331
 *
 * Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 30.5 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 31.11960

```

* Volume of runoff (Step 4) V= 2.272951
386 UI 1.803 3.955 6.546 9.238 12.026 14.983 17.372 20.000 24.055 27.569
387 UI 30.893 34.920 39.041 43.238 47.883 52.555 57.259 62.759 68.502 74.530
388 UI 80.144 84.734 86.058 86.991 87.571 86.838 85.782 84.458 80.116 75.689
389 UI 71.182 66.627 62.247 58.247 55.395 52.894 50.416 47.642 44.826 43.023
390 UI 41.175 39.288 37.345 35.609 34.144 32.655 31.216 29.892 28.547 27.283
391 UI 26.354 25.314 24.238 23.165 22.476 21.575 20.757 20.000 19.325 18.722
392 UI 18.095 17.391 16.794 16.242 15.735 15.228 14.735 14.368 13.896 13.333
393 UI 13.018 12.625 12.118 11.856 11.577 11.183 10.959 10.768 10.486 10.259
394 UI 10.057 9.803 9.550 9.296 9.070 8.869 8.700 8.482 8.266 8.068
395 UI 7.914 7.762 7.565 7.367 7.177 7.065 6.907 6.710 6.595 6.447
396 UI 6.250 6.150 6.031 5.834 5.700 5.582 5.441 5.354 5.289 5.120
397 UI 4.981 4.868 4.755 4.643 4.530 4.441 4.357 4.272 4.167 4.060
398 UI 3.975 3.891 3.806 3.722 3.637 3.553 3.468 3.384 3.299 3.239
399 UI 3.166 3.053 2.982 2.920 2.836 2.769 2.713 2.657 2.600 2.544
400 UI 2.488 2.431 2.375 2.319 2.262 2.206 2.170 2.142 2.114 2.068
401 UI 2.014 1.986 1.943 1.887 1.830 1.785 1.757 1.729 1.693 1.636
402 UI 1.601 1.568 1.511 1.472 1.444 1.416 1.388 1.360 1.332 1.303
403 UI 1.275 1.247 1.219 1.191 1.163 1.135 1.106 1.078 1.052 1.052
404 UI 1.038 1.009 0.981 0.953 0.925 0.920
405 ZW C=FLOW P=010YR-10DY A=GREENBRI
*

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```

406 KK J
* JXMIN Time interval for input data
407 IN 60
408 KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
409 PB 0
410 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
411 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
412 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
413 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
414 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
415 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
416 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
417 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
418 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
419 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
420 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
421 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
422 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
423 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
424 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
425 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
426 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
427 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
428 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
429 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
430 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
431 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
432 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
433 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
434 BA 0.0859
* STRTL CNSTL RTIMP
435 LU 0.20 0.056 75.173
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 19.3 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 19.84462

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* Volume of runoff (Step 4) V= 2.310764
436 UI 4.52 11.00 17.78 25.11 30.86 40.90 49.10 59.20 69.77 81.40
437 UI 93.35 107.46 122.39 134.99 138.13 139.50 137.48 133.10 122.14 110.85
438 UI 99.67 90.31 84.06 77.65 70.88 66.33 61.58 56.98 53.31 49.67
439 UI 46.35 43.23 40.80 38.20 36.00 33.80 31.86 30.23 28.67 27.02
440 UI 25.66 24.39 23.24 22.16 20.97 20.02 19.01 18.29 17.52 16.98
441 UI 16.36 15.81 15.18 14.58 14.07 13.59 13.06 12.63 12.21 11.72
442 UI 11.32 10.94 10.55 10.16 9.81 9.46 9.08 8.76 8.52 8.22
443 UI 7.88 7.60 7.32 7.07 6.86 6.61 6.37 6.16 5.95 5.74
444 UI 5.53 5.31 5.15 4.92 4.73 4.54 4.38 4.24 4.09 3.95
445 UI 3.81 3.67 3.53 3.44 3.37 3.25 3.16 3.04 2.90 2.81
446 UI 2.74 2.61 2.53 2.40 2.32 2.25 2.18 2.11 2.04 1.97
447 UI 1.90 1.83 1.75 1.68 1.68 1.61 1.54 1.47 1.47
448 ZW C=FLOW F=010YR-10DY A=GREENBRI

```

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HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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449 KK OFF-N
* JXMIN Time interval for input data
450 IN 60
451 KM E5
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Area adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-6 of total rainfall: 7.4646
452 PB 0
453 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
454 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
455 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
456 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0149 0.0224
457 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
458 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
459 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
460 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
461 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
462 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
463 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
464 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
465 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
466 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
467 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
468 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
469 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
470 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
471 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
472 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
473 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
474 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
475 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075

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HEC-1L INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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476 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
477 BA 0.8438
* STRTL CNSTL RTIMP
478 LU 0.20 0.070 2.000
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 78.2 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 78.67958
* Volume of runoff (Step 4) V= 22.687500
479 UI 2.81 5.63 8.44 11.34 15.39 19.43 23.48 27.55 31.77 35.99
480 UI 40.21 44.51 49.13 53.75 58.37 62.68 66.16 69.63 73.11 77.52

```


481	UI	83.86	90.19	96.52	102.35	107.41	112.47	117.52	123.15	129.44	135.73
482	UI	142.02	148.44	155.00	161.55	168.10	175.07	182.33	189.59	196.84	204.16
483	UI	211.50	218.85	226.19	234.56	243.31	252.06	260.81	270.09	279.50	288.91
484	UI	298.32	307.02	315.59	324.17	332.74	341.30	349.87	358.44	367.01	375.58
485	UI	343.80	344.94	345.96	344.82	343.67	342.53	341.27	339.21	337.14	335.07
486	UI	332.11	325.29	318.48	311.66	304.79	297.75	290.71	283.68	276.61	269.49
487	UI	262.36	255.24	248.45	242.21	235.96	229.72	224.53	220.61	216.70	212.79
488	UI	208.89	205.02	201.15	197.28	193.00	188.43	183.86	179.28	175.85	173.04
489	UI	170.22	167.41	164.50	161.55	158.61	155.66	152.64	149.61	146.57	143.54
490	UI	141.14	138.85	136.57	134.28	131.95	129.62	127.29	124.96	122.89	120.82
491	UI	118.75	116.68	114.57	112.46	110.35	108.31	106.86	105.41	103.96	102.46
492	UI	100.75	99.03	97.32	95.69	94.32	92.96	91.60	90.22	88.81	87.41
493	UI	86.00	84.64	83.37	82.09	80.82	79.64	78.58	77.53	76.47	75.48
494	UI	74.56	73.64	72.71	71.69	70.59	69.49	68.39	67.43	66.55	65.67
495	UI	64.79	63.97	63.18	62.39	61.60	60.81	60.02	59.22	58.43	57.83
496	UI	57.25	56.68	56.11	55.26	54.38	53.50	52.62	52.13	51.64	51.16
497	UI	50.66	49.87	49.08	48.29	47.54	47.19	46.84	46.49	46.09	45.47
498	UI	44.86	44.24	43.71	43.45	43.18	42.92	42.60	42.16	41.72	41.28
499	UI	40.89	40.58	40.28	39.97	39.62	39.23	38.83	38.43	38.04	37.64
500	UI	37.25	36.85	36.48	36.13	35.78	35.42	35.13	34.86	34.60	34.34
501	UI	34.01	33.66	33.31	32.96	32.64	32.33	32.02	31.71	31.48	31.26
502	UI	31.04	30.82	30.52	30.21	29.90	29.60	29.29	28.98	28.67	28.37
503	UI	28.19	28.02	27.84	27.65	27.35	27.04	26.73	26.44	26.27	26.09
504	UI	25.92	25.71	25.40	25.09	24.79	24.53	24.40	24.27	24.13	23.94
505	UI	23.63	23.32	23.02	22.76	22.59	22.41	22.24	22.04	21.82	21.60
506	UI	21.38	21.23	21.14	21.06	20.97	20.77	20.51	20.24	19.98	19.78
507	UI	19.60	19.42	19.25	19.07	18.90	18.72	18.54	18.37	18.19	18.02
508	UI	17.84	17.70	17.57	17.44	17.31	17.18	17.04	16.91	16.78	16.60
509	UI	16.43	16.25	16.08	15.95	15.82	15.68	15.55	15.42	15.29	15.16
510	UI	15.02	14.89	14.76	14.63	14.50	14.36	14.23	14.10	13.97	13.84
511	UI	13.70	13.57	13.44	13.31	13.18	13.04	12.93	12.85	12.76	12.67
512	UI	12.53	12.36	12.18	12.01	11.88	11.80	11.71	11.62	11.50	11.37
513	UI	11.24	11.11	11.01	10.92	10.83	10.74	10.66	10.57	10.48	10.39
514	UI	10.30	10.22	10.13	10.04	9.95	9.86	9.78	9.69	9.60	9.51
515	UI	9.42	9.34	9.25	9.16	9.07	8.98	8.90	8.81	8.72	8.64
516	UI	8.60	8.55	8.51	8.47	8.42	8.38	8.33	8.27	8.19	8.10
517	UI	8.01	7.94	7.90	7.85	7.81	7.74	7.66	7.57	7.48	7.39
518	UI	7.30	7.22	7.13	7.07	7.02	6.98	6.94	6.89	6.85	6.80

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LINE	ID	1	2	3	4	5	6	7	8	9	10
519	UI	6.76	6.68	6.59	6.51	6.42	6.37	6.32	6.28	6.23	6.15
520	UI	6.06	5.98	5.89	5.84	5.80	5.75	5.71	5.67	5.62	5.58
521	UI	5.53	5.49	5.45	5.40	5.36	5.31	5.27	5.23	5.18	5.14
522	UI	5.09	5.05	5.01	4.96	4.92	4.87	4.83	4.79	4.74	4.70
523	UI	4.65	4.61	4.57	4.52	4.48	4.43	4.39	4.35	4.30	4.26
524	UI	4.21	4.17	4.15	4.15	4.15	4.15	4.12	4.08	4.04	3.99
525	UI	3.95	3.90	3.86	3.82	3.77	3.73	3.68	3.64	3.63	3.63
526	UI	3.63	3.63								
527	ZW	C=FLOW F=010YR-10DY A=GREENBRI									
	*										
528	KK	DUMMY2									
529	KM										
530	HC	6									
531	ZW	C=FLOW									
	*										
532	KK	X									
	*	JXMIN Time interval for input data									
533	IN	60									
534	KM										
	*	Design storm construction details									
	*	Regional multiplier (zone 2) applied: 1.000									
	*	Areal adjustment using area: 1.813									
	*	multiplier from table 4-4: 1.0000									
	*	Adjusted depths for each duration from table 4-1; frequency: 10									
	*	Duration---Regional---Elev---Areal (adjustments)									
	*	5 min	0.2500	0.2500	0.2500						
	*	10 min	0.3600	0.3600	0.3600						
	*	15 min	0.4300	0.4300	0.4300						
	*	30 min	0.5700	0.5700	0.5700						
	*	1 hour	0.7700	0.7700	0.7700						
	*	2 hours	1.0400	1.0400	1.0315						
	*	3 hours	1.2300	1.2300	1.2200						
	*	6 hours	1.6500	1.6500	1.6366						
	*	12 hours	2.2500	2.2500	2.2500						
	*	24 hours	2.9800	2.9800	2.9800						
	*	36 hours	3.5400	3.5400	3.5046						
	*	2 days	3.9500	3.9500	3.9500						
	*	3 days	4.6500	4.6500	4.6500						
	*	5 days	5.7600	5.7600	5.7024						
	*	10 days	7.5400	7.5400	7.4646						
	*	Storm duration: 10, length: 240 ordinates									
	*	Distribution using table 4-8 of total rainfall: 7.4646									
535	PB	0									
536	PI	0.0224	0.0821	0.2015	0.1120	0.0373	0.0224	0.0075	0.0000	0.0000	0.0000
537	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
538	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
539	PI	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0075	0.0149	0.0224
540	PI	0.0373	0.0523	0.0672	0.0970	0.2239	0.1418	0.0746	0.0597	0.0448	0.0373

541 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 542 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

543 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 544 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 545 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
 546 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
 547 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
 548 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
 549 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
 550 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
 551 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
 552 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 553 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 554 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
 555 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
 556 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 557 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 558 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
 559 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149

* Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TAREA subbasin area (sq mi)

560 BA 0.0630
 * STRL CNSTL RTIMP
 561 LU 0.20 0.064 47.945

* Runoff hydrograph computation (Chapter 5)
 * Using basin lag: 24.8 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 25.30745
 * Volume of runoff (Step 4) V_r 1.693160

562 UI 2.030 4.713 7.691 10.795 14.126 16.683 20.577 24.770 28.545 33.081
 563 UI 37.785 42.822 48.078 53.458 59.771 66.472 72.929 77.779 79.257 80.081
 564 UI 79.662 78.512 76.256 71.339 66.270 61.150 56.282 52.049 49.226 46.431
 565 UI 43.319 40.484 38.444 36.318 34.140 32.258 30.599 28.925 27.434 25.918
 566 UI 24.637 23.544 22.307 21.318 20.312 19.339 18.448 17.686 17.012 16.264
 567 UI 15.532 14.905 14.334 13.763 13.285 12.801 12.183 11.834 11.309 10.905
 568 UI 10.602 10.157 9.966 9.683 9.410 9.175 8.890 8.604 8.340 8.112
 569 UI 7.913 7.659 7.433 7.250 7.069 6.847 6.625 6.480 6.301 6.106
 570 UI 5.978 5.756 5.629 5.472 5.271 5.144 4.987 4.894 4.777 4.596
 571 UI 4.469 4.342 4.216 4.100 4.005 3.910 3.788 3.679 3.584 3.489
 572 UI 3.394 3.299 3.203 3.108 3.013 2.949 2.840 2.742 2.673 2.577
 573 UI 2.511 2.448 2.384 2.321 2.257 2.194 2.131 2.067 2.005 1.974
 574 UI 1.942 1.894 1.838 1.807 1.743 1.680 1.631 1.599 1.568 1.506
 575 UI 1.464 1.420 1.361 1.329 1.297 1.265 1.234 1.202 1.170 1.138
 576 UI 1.107 1.075 1.043 1.012 0.980 0.963 0.957 0.925 0.893 0.861
 577 UI 0.843
 578 ZW C=FLOW F=010YR-10DY A=GREENBRI

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

579 KK L
 * JKMIN Time interval for input data
 580 IN 60
 581 KM
 * Design storm construction details
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 1.813
 * multiplier from table 4-4: 1.0000
 * Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration-----Regional-----Elev-----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2500
 * 10 min 0.3600 0.3600 0.3600
 * 15 min 0.4300 0.4300 0.4300
 * 30 min 0.5700 0.5700 0.5700
 * 1 hour 0.7700 0.7700 0.7700
 * 2 hours 1.0400 1.0400 1.0315
 * 3 hours 1.2300 1.2300 1.2200
 * 6 hours 1.6500 1.6500 1.6366
 * 12 hours 2.2500 2.2500 2.2500
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646
 582 PB 0
 583 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
 584 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 585 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 586 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224

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587 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
588 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
589 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
590 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
591 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
592 PI 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448
593 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
594 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
595 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
596 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
597 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
598 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
599 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
600 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
601 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
602 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
603 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
604 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
605 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075

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1

HEC-1L INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
606 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
607 BA 0.0330
* STREL CNSTL RTIMP
608 LU 0.20 0.060 70.000
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 10.8 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 11.25192
* Volume of runoff (Step 4) V= 0.886493
609 UI 6.407 14.667 22.873 33.597 45.864 59.705 76.233 91.581 94.540 91.593
610 UI 79.589 66.311 57.178 49.115 43.425 38.013 33.648 29.652 26.656 23.939
611 UI 21.516 19.636 17.744 16.220 14.903 13.668 12.625 11.742 11.022 10.309
612 UI 9.615 9.027 8.495 7.940 7.493 7.046 6.604 6.137 5.784 5.418
613 UI 5.081 4.776 4.504 4.225 3.973 3.721 3.494 3.232 3.016 2.848
614 UI 2.680 2.512 2.354 2.270 2.139 1.982 1.876 1.745 1.619 1.529
615 UI 1.445 1.361 1.277 1.193 1.135 1.072 0.993
616 ZW C=FLOW P=010YR-10DY A=GREENBRI
*
KK M
* JXMIN Time interval for input data
618 IN 60
619 KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional---Elev---Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2800 2.2800 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646

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1

HEC-1L INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
620 PB 0
621 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
622 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
623 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
624 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
625 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
626 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
627 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
628 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
629 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
630 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
631 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
632 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000

```

633 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
634 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
635 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
636 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
637 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
638 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
639 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
640 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
641 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
642 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
643 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
644 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149

*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)

645 BA 0.0666
* STRL CNSTL RTIMP
646 LU 0.20 0.040 95.000

*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 26.3 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 26.79198
* Volume of runoff (Step 4) V= 1.789792

647 UI 1.915 4.383 7.174 10.047 13.183 15.826 18.777 23.086 26.559 30.522
648 UI 34.851 39.310 44.219 49.193 54.442 60.397 66.766 72.836 77.583 78.990
649 UI 79.827 79.723 78.803 77.397 73.123 68.404 63.604 58.757 54.429 51.019
650 UI 48.362 45.729 42.689 40.188 38.257 36.253 34.198 32.383 30.823 29.237
651 UI 27.797 26.377 25.000 24.013 22.883 21.825 20.894 19.936 19.048 18.246
652 UI 17.536 16.908 16.189 15.503 14.909 14.371 13.832 13.353 12.952 12.354
653 UI 11.949 11.540 11.013 10.774 10.409 10.076 9.894 9.595 9.357 9.127
654 UI 8.858 8.588 8.339 8.119 7.939 7.701 7.481 7.290 7.140 6.933
655 UI 6.723 6.539 6.420 6.215 6.060 5.917 5.707 5.609 5.449 5.261
656 UI 5.142 4.995 4.896 4.809 4.629 4.503 4.383 4.264 4.144 4.050
657 UI 3.961 3.865 3.745 3.651 3.561 3.472 3.382 3.292 3.202 3.113
658 UI 3.023 2.957 2.869 2.758 2.698 2.614 2.538 2.478 2.418 2.358
659 UI 2.299 2.239 2.179 2.119 2.059 2.002 1.972 1.942 1.900 1.842
660 UI 1.812 1.761 1.701 1.642 1.612 1.582 1.542 1.482 1.452 1.402
661 UI 1.353 1.323 1.293 1.263 1.233 1.203 1.173 1.143 1.113 1.083
662 UI 1.053 1.023 0.993 0.964 0.962 0.944 0.914 0.884 0.854 0.842

HEC-1L INPUT

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

663 ZW C=FLOW F=010YR-10DY A=GREENBRI

664 KK N
* JXMIN Time interval for input data

665 IN 60

666 KM

* Design storm construction details

* Regional multiplier (zone 2) applied: 1.000

* Areal adjustment using area: 1.813

* multiplier from table 4-4: 1.0000

* Adjusted depths for each duration from table 4-1: frequency: 10

* Duration---Regional-----Elev-----Areal (adjustments)

* 5 min 0.2500 0.2500 0.2500

* 10 min 0.3600 0.3600 0.3600

* 15 min 0.4300 0.4300 0.4300

* 30 min 0.5700 0.5700 0.5700

* 1 hour 0.7700 0.7700 0.7700

* 2 hours 1.0400 1.0400 1.0315

* 3 hours 1.2300 1.2300 1.2200

* 6 hours 1.6500 1.6500 1.6366

* 12 hours 2.2500 2.2500 2.2500

* 24 hours 2.9800 2.9800 2.9800

* 36 hours 3.5400 3.5400 3.5046

* 2 days 3.9500 3.9500 3.9500

* 3 days 4.6500 4.6500 4.6500

* 5 days 5.7600 5.7600 5.7024

* 10 days 7.5400 7.5400 7.4646

* Storm duration: 10, length: 240 ordinates

* Distribution using table 4-8 of total rainfall: 7.4646

667 FB 0
668 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
669 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
670 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
671 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
672 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
673 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
674 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
675 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
676 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
677 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
678 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
679 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
680 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
681 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
682 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672

683 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
 684 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 685 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 686 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
 687 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
 688 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

HEC-1L INPUT

PAGE 25

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

689 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 690 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
 691 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149

*
 * Precipitation losses computation (Chapter 5)
 * Computing RTIMP (percent impervious) from land use and table 5-2
 * Computing CNSTL (infiltration rate) from soil type and table 5-2
 * TAREA subbasin area (sq mi)
 692 BA 0.0375
 * SRTL CNSTL RTIMP
 693 LU 0.20 0.068 43.112

*
 * Runoff hydrograph computation (Chapter 6)
 * Using basin lag: 19.1 minutes
 * Using unit duration (Step 2): 1. min
 * Lag Time + Unit Duration / 2 (Step 3): 19.61394
 * Volume of runoff (Step 4) V= 1.008333

694 UI 2.030 4.928 7.964 11.204 13.886 18.309 22.045 26.572 31.346 36.548
 695 UI 42.014 48.382 54.963 59.839 61.126 61.438 60.404 57.745 52.812 47.756
 696 UI 42.915 39.157 36.372 33.371 30.707 28.647 26.506 24.625 22.972 21.414
 697 UI 19.917 18.699 17.541 16.479 15.483 14.542 13.742 13.057 12.301 11.637
 698 UI 11.057 10.491 10.056 9.460 9.076 8.535 8.254 7.828 7.625 7.315
 699 UI 7.093 6.810 6.529 6.281 6.087 5.839 5.626 5.460 5.240 5.036
 700 UI 4.892 4.692 4.544 4.356 4.227 4.035 3.899 3.773 3.666 3.501
 701 UI 3.375 3.249 3.137 3.042 2.933 2.823 2.728 2.634 2.540 2.445
 702 UI 2.351 2.276 2.174 2.088 2.004 1.931 1.868 1.806 1.743 1.680
 703 UI 1.617 1.554 1.517 1.485 1.427 1.391 1.332 1.269 1.235 1.204
 704 UI 1.142 1.110 1.048 1.016 0.985 0.954 0.922 0.891 0.859 0.828
 705 UI 0.796 0.765 0.740 0.733 0.701 0.670 0.648
 706 ZW C=FLOW F=010YR-10DY A=GREENBRI

707 KK 0
 * JXMIN Time interval for input data
 708 IN 60
 709 KM

*
 * Design storm construction details
 *
 * Regional multiplier (zone 2) applied: 1.000
 * Areal adjustment using area: 1.813
 * multiplier from table 4-4: 1.0000

* Adjusted depths for each duration from table 4-1: frequency: 10
 * Duration---Regional-----Elev-----Areal (adjustments)
 * 5 min 0.2500 0.2500 0.2500
 * 10 min 0.3600 0.3600 0.3600
 * 15 min 0.4300 0.4300 0.4300
 * 30 min 0.5700 0.5700 0.5700
 * 1 hour 0.7700 0.7700 0.7700
 * 2 hours 1.0400 1.0400 1.0315
 * 3 hours 1.2300 1.2300 1.2200
 * 6 hours 1.6500 1.6500 1.6366
 * 12 hours 2.2500 2.2500 2.2500
 * 24 hours 2.9800 2.9800 2.9800
 * 36 hours 3.5400 3.5400 3.5046
 * 2 days 3.9500 3.9500 3.9500
 * 3 days 4.6500 4.6500 4.6500
 * 5 days 5.7600 5.7600 5.7024
 * 10 days 7.5400 7.5400 7.4646
 * Storm duration: 10, length: 240 ordinates
 * Distribution using table 4-8 of total rainfall: 7.4646

HEC-1L INPUT

PAGE 26

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

710 PB 0
 711 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
 712 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 713 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 714 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
 715 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
 716 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 717 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 718 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 719 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 720 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
 721 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
 722 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
 723 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0075
 724 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
 725 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
 726 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
 727 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 728 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

```

729 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0521 0.0672 0.0970 0.2911 0.1493
730 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
731 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
732 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
733 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
734 PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
735 BA 0.0402
* STRL CNSTL RTIMP
736 LU 0.20 0.070 50.000
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 15.2 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 15.66410
* Volume of runoff (Step 4) V= 1.079757
737 UI 3.700 8.650 14.029 18.571 25.815 32.653 40.437 49.127 58.596 69.581
738 UI 79.786 81.989 82.075 79.942 72.282 63.797 55.826 50.481 45.662 41.057
739 UI 37.576 34.011 31.250 28.611 26.091 24.250 22.347 20.679 19.122 17.893
740 UI 16.687 15.545 14.582 13.717 12.831 12.134 11.294 10.743 10.266 9.781
741 UI 9.383 8.907 8.469 8.130 7.726 7.417 7.070 6.735 6.454 6.189
742 UI 5.858 5.635 5.348 5.098 4.949 4.673 4.462 4.253 4.095 3.911
743 UI 3.737 3.578 3.420 3.261 3.103 2.975 2.809 2.669 2.556 2.450
744 UI 2.345 2.239 2.134 2.048 1.995 1.901 1.835 1.730 1.660 1.601
745 UI 1.513 1.431 1.366 1.313 1.260 1.207 1.155 1.102 1.049 0.996
746 UI 0.985 0.932 0.879
747 ZW C=FLOW P=010YR-10DY A=GREENBRI
*

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HEC-1L INPUT

1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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```

748 KK P
* JXMIN Time interval for input data
749 IN 60
750 KM
*
* Design storm construction details
*
* Regional multiplier (zone 2) applied: 1.000
* Areal adjustment using area: 1.813
* multiplier from table 4-4: 1.0000
*
* Adjusted depths for each duration from table 4-1: frequency: 10
* Duration---Regional-----Elev-----Areal (adjustments)
* 5 min 0.2500 0.2500 0.2500
* 10 min 0.3600 0.3600 0.3600
* 15 min 0.4300 0.4300 0.4300
* 30 min 0.5700 0.5700 0.5700
* 1 hour 0.7700 0.7700 0.7700
* 2 hours 1.0400 1.0400 1.0315
* 3 hours 1.2300 1.2300 1.2200
* 6 hours 1.6500 1.6500 1.6366
* 12 hours 2.2500 2.2500 2.2500
* 24 hours 2.9800 2.9800 2.9800
* 36 hours 3.5400 3.5400 3.5046
* 2 days 3.9500 3.9500 3.9500
* 3 days 4.6500 4.6500 4.6500
* 5 days 5.7600 5.7600 5.7024
* 10 days 7.5400 7.5400 7.4646
* Storm duration: 10, length: 240 ordinates
* Distribution using table 4-8 of total rainfall: 7.4646
751 PB 0
752 PI 0.0224 0.0821 0.2015 0.1120 0.0373 0.0224 0.0075 0.0000 0.0000 0.0000
753 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
754 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
755 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224
756 PI 0.0373 0.0523 0.0672 0.0970 0.2239 0.1418 0.0746 0.0597 0.0448 0.0373
757 PI 0.0299 0.0224 0.0149 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
758 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
759 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
760 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
761 PI 0.0000 0.0000 0.0000 0.0075 0.0149 0.0224 0.0299 0.0373 0.0448 0.0523
762 PI 0.0672 0.1120 0.3956 0.1642 0.0746 0.0597 0.0448 0.0373 0.0299 0.0224
763 PI 0.0224 0.0149 0.0149 0.0075 0.0075 0.0000 0.0000 0.0000 0.0000 0.0000
764 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075
765 PI 0.0149 0.0149 0.0149 0.0224 0.0373 0.0448 0.0523 0.0672 0.0746 0.0821
766 PI 0.0970 0.1045 0.1194 0.1269 0.1344 0.1418 0.1568 0.1120 0.0896 0.0672
767 PI 0.2314 0.2911 0.5001 0.2463 0.0373 0.0224 0.0149 0.0075 0.0075 0.0075
768 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
769 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
770 PI 0.0075 0.0149 0.0224 0.0299 0.0373 0.0523 0.0672 0.0970 0.2911 0.1493
771 PI 0.0746 0.0597 0.0523 0.0448 0.0373 0.0299 0.0224 0.0149 0.0075 0.0000
772 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
773 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
774 PI 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0075

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HEC-1L INPUT

1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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775      PI 0.0149 0.0373 0.0523 0.0746 0.2165 0.1194 0.0597 0.0448 0.0299 0.0149
*
* Precipitation losses computation (Chapter 5)
* Computing RTIMP (percent impervious) from land use and table 5-2
* Computing CNSTL (infiltration rate) from soil type and table 5-2
* TAREA subbasin area (sq mi)
776      BA 0.0342
* STRTL CNSTL RTIMP
777      LU 0.20 0.062 56.384
*
* Runoff hydrograph computation (Chapter 6)
* Using basin lag: 16.5 minutes
* Using unit duration (Step 2): 1. min
* Lag Time + Unit Duration / 2 (Step 3): 17.01644
* Volume of runoff (Step 4) V= 0.920104
778      UI 2.600 6.153 9.967 13.380 18.241 22.789 28.286 34.177 40.506 47.656
779      UI 55.696 62.676 64.278 64.502 63.082 58.468 52.398 46.323 41.479 38.101
780      UI 34.392 31.539 28.996 26.479 24.484 22.572 20.756 19.368 17.921 16.730
781      UI 15.552 14.546 13.707 12.781 12.021 11.335 10.728 10.081 9.564 8.918
782      UI 8.577 8.137 7.838 7.527 7.202 6.861 6.571 6.316 6.030 5.813
783      UI 5.561 5.305 5.124 4.903 4.688 4.527 4.284 4.125 3.968 3.824
784      UI 3.641 3.488 3.337 3.223 3.095 2.962 2.847 2.733 2.618 2.504
785      UI 2.404 2.287 2.187 2.079 2.002 1.925 1.849 1.773 1.697 1.621
786      UI 1.583 1.533 1.474 1.413 1.336 1.295 1.249 1.186 1.129 1.078
787      UI 1.040 1.001 0.963 0.925 0.887 0.849 0.811 0.779 0.767 0.729
788      UI 0.691 0.681
789      ZW C=FLOW P=010YR-10DY A=GREENBRI
*

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```

790      KK DUMMY3
791      KM
792      HC 6
793      ZW C=FLOW
794      ZZ

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```

*****
* FLOOD HYDROGRAPH PACKAGE (REC-1L) *
* JULY 1998 *
* VERSION 4.1.1(L) *
* RUN DATE 12JUL05 TIME 19:20:27 *
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

10010 & 1010 & 10024&1024

```

3 IO      OUTPUT CONTROL VARIABLES
          IPRINT 3 PRINT CONTROL
          IPLOT 0 PLOT CONTROL
          QSCAL 0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN 1 MINUTES IN COMPUTATION INTERVAL
          IDATE 31DEC99 STARTING DATE
          ITIME 0000 STARTING TIME
          NQ 14947 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE 11JAN 0 ENDING DATE
          NDTIME 0906 ENDING TIME
          ICENT 19 CENTURY MARK

          COMPUTATION INTERVAL 0.02 HOURS
          TOTAL TIME BASE 249.10 HOURS

```

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ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

```

*** ** ** ** **

```

*****
* A *
*****

```

```

5 IN      TIME DATA FOR INPUT TIME SERIES
          JXMIN 60 TIME INTERVAL IN MINUTES
          JXDATE 31DEC99 STARTING DATE
          JXTIME 2400 STARTING TIME

```

SUBBASIN RUNOFF DATA

32 BA SUBBASIN CHARACTERISTICS
TAREA. 0.12 SUBBASIN AREA

PRECIPITATION DATA

7 PB STORM 7.46 BASIN TOTAL PRECIPITATION

33 LU UNIFORM LOSS RATE

STRTL 0.20 INITIAL LOSS
CNSTL 0.06 UNIFORM LOSS RATE
RTIMP 60.15 PERCENT IMPERVIOUS AREA

32 UI INPUT UNITGRAPH, 173 ORDINATES, VOLUME = 1.00

2.9	6.5	10.7	15.1	19.7	24.2	27.8	33.7	39.7	45.0
51.3	57.9	64.7	72.2	79.7	87.5	96.5	106.0	115.5	124.4
127.7	129.5	130.7	129.7	128.0	125.7	118.6	111.5	104.2	96.9
90.2	84.4	80.4	76.4	72.1	67.4	64.4	61.4	58.4	55.2
52.6	50.2	47.8	45.6	43.5	41.3	39.7	38.1	36.3	34.9
33.5	32.0	30.7	29.6	28.5	27.5	26.5	25.4	24.5	23.7
22.9	22.0	21.5	20.7	19.8	19.3	18.6	17.9	17.5	17.0
16.5	16.2	15.7	15.4	15.0	14.6	14.2	13.8	13.4	13.1
12.9	12.5	12.2	11.9	11.6	11.3	11.0	10.7	10.5	10.3
10.0	9.8	9.5	9.3	9.1	8.8	8.6	8.4	8.1	8.0
7.9	7.6	7.4	7.2	7.0	6.9	6.7	6.6	6.4	6.3
6.1	5.9	5.8	5.7	5.5	5.4	5.3	5.1	5.0	4.9
4.8	4.6	4.5	4.4	4.2	4.1	4.0	4.0	3.9	3.8
3.7	3.6	3.5	3.4	3.3	3.3	3.2	3.2	3.1	3.0
3.0	2.9	2.8	2.7	2.6	2.6	2.5	2.5	2.4	2.3
2.3	2.2	2.2	2.1	2.1	2.0	2.0	1.9	1.9	1.8
1.8	1.7	1.7	1.6	1.6	1.6	1.6	1.5	1.5	1.4
1.4	1.4	1.4							

*** **

HYDROGRAPH AT STATION A

TOTAL RAINFALL = 7.46, TOTAL LOSS = 1.72, TOTAL EXCESS = 5.74

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			249.10-HR
		6-HR	24-HR	72-HR	
33.	153.07	16.	7.	4.	2.
		(INCHES) 1.245	2.366	3.390	5.736
		(AC-FT) 8.	15.	21.	36.

CUMULATIVE AREA = 0.12 SQ MI

* *
53 KK * B *
* *

54 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 60 TIME INTERVAL IN MINUTES
JXDATE 31DEC99 STARTING DATE
JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

81 BA SUBBASIN CHARACTERISTICS
TAREA. 0.04 SUBBASIN AREA

PRECIPITATION DATA

56 PB STORM 7.46 BASIN TOTAL PRECIPITATION

82 LU UNIFORM LOSS RATE

STRTL 0.20 INITIAL LOSS
CNSTL 0.06 UNIFORM LOSS RATE
RTIMP 57.84 PERCENT IMPERVIOUS AREA

81 UI INPUT UNITGRAPH, 91 ORDINATES, VOLUME = 1.00

4.3	9.9	16.1	21.7	29.6	37.7	46.8	56.8	68.1	80.8
89.4	91.5	90.4	85.6	76.0	66.3	58.6	53.3	47.2	43.3
39.2	35.8	32.6	29.8	27.5	25.2	23.3	21.5	20.0	18.7
17.3	16.2	15.2	14.2	13.4	12.5	11.8	11.3	10.8	10.3
9.8	9.3	8.9	8.4	8.1	7.7	7.4	7.0	6.7	6.4
6.1	5.8	5.6	5.3	5.0	4.8	4.6	4.4	4.2	4.0
3.8	3.6	3.5	3.3	3.1	3.0	2.8	2.7	2.6	2.5
2.4	2.3	2.2	2.1	2.0	1.9	1.8	1.7	1.7	1.6
1.5	1.4	1.4	1.3	1.3	1.2	1.1	1.1	1.1	1.0
1.0									


```

***
***
***
***
***
HYDROGRAPH AT STATION B
TOTAL RAINFALL = 7.46, TOTAL LOSS = 1.84, TOTAL EXCESS = 5.63
PEAK FLOW      TIME
+ (CFS)        (HR)
+ 13.          153.00
(CFS)
(INCHES)      6.          3.          1.          1.
              1.251     2.328     3.329     5.617
(AC-FT)       3.          5.          8.          13.
CUMULATIVE AREA = 0.04 SQ MI

```

```

*****
*
*
94 KK * C
*
*
*****

```

```

95 IN TIME DATA FOR INPUT TIME SERIES
      JXMIN      60 TIME INTERVAL IN MINUTES
      JXDATE     31DEC99 STARTING DATE
      JXTIME     2400 STARTING TIME

```

```

SUBBASIN RUNOFF DATA
122 BA SUBBASIN CHARACTERISTICS
      TAREA,     0.06 SUBBASIN AREA

```

```

PRECIPITATION DATA
97 FB STORM      7.46 BASIN TOTAL PRECIPITATION

```

```

123 LU UNIFORM LOSS RATE
      STRL      0.20 INITIAL LOSS
      CNSTL     0.06 UNIFORM LOSS RATE
      RTIMP     56.96 PERCENT IMPERVIOUS AREA

```

```

122 UI INPUT UNITGRAPH, 114 ORDINATES, VOLUME = 1.00
      3.6      8.6      13.9      19.4      24.6      32.0      38.8      46.7      55.2      64.3
      74.3     85.7     96.7     102.1     103.8     103.1     100.8     93.3     84.5     75.7
      68.0     63.1     58.2     52.9     49.4     45.7     42.2     39.3     36.6     34.0
      31.8     29.8     27.9     26.2     24.5     23.2     22.0     20.6     19.5     18.5
      17.5     16.8     15.8     15.1     14.2     13.7     13.1     12.7     12.2     11.8
      11.3     10.8     10.4     10.0     9.6      9.3      9.0      8.6      8.4      8.0
      7.8      7.4      7.2      6.8      6.6      6.4      6.2      5.9      5.7      5.5
      5.3      5.1      4.9      4.8      4.6      4.4      4.3      4.1      3.9      3.8
      3.6      3.5      3.3      3.2      3.1      3.0      2.9      2.8      2.7      2.6
      2.5      2.5      2.4      2.3      2.2      2.1      2.0      2.0      1.9      1.8
      1.7      1.7      1.6      1.6      1.5      1.5      1.4      1.3      1.3      1.3
      1.2      1.2      1.1      1.1

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HYDROGRAPH AT STATION C
TOTAL RAINFALL = 7.46, TOTAL LOSS = 1.89, TOTAL EXCESS = 5.57
PEAK FLOW      TIME
+ (CFS)        (HR)
+ 18.          153.02
(CFS)
(INCHES)      8.          4.          2.          1.
              1.241     2.310     3.301     5.563
(AC-FT)       4.          8.          11.         18.
CUMULATIVE AREA = 0.06 SQ MI

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137 KK * D
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138 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

165 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.05 SUBBASIN AREA

PRECIPITATION DATA

140 PB STORM 7.46 BASIN TOTAL PRECIPITATION

166 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 47.04 PERCENT IMPERVIOUS AREA

165 UI INPUT UNITGRAPH, 111 ORDINATES, VOLUME = 1.00

2.9	6.9	11.2	15.4	19.9	25.6	31.2	37.6	44.5	51.9
50.1	69.3	77.9	79.9	80.7	79.4	76.0	69.2	62.2	55.6
50.7	46.9	42.5	39.4	36.5	33.5	31.2	28.9	26.9	25.0
23.4	21.9	20.5	19.2	18.1	17.1	16.1	15.2	14.4	13.6
13.0	12.2	11.6	11.0	10.5	10.1	9.7	9.4	9.0	8.6
8.3	8.0	7.7	7.4	7.1	6.8	6.6	6.3	6.1	5.9
5.7	5.4	5.2	5.0	4.9	4.6	4.5	4.3	4.1	4.0
3.9	3.7	3.6	3.4	3.3	3.2	3.0	3.0	2.8	2.7
2.6	2.5	2.4	2.3	2.2	2.1	2.0	2.0	1.9	1.9
1.8	1.7	1.7	1.6	1.6	1.5	1.4	1.4	1.3	1.3
1.2	1.2	1.1	1.1	1.0	1.0	1.0	1.0	0.9	0.9
0.8									

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HYDROGRAPH AT STATION D

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.41, TOTAL EXCESS = 5.06

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	249.10-HR	
14.	153.02	6.	3.	1.	1.	
		(INCHES)	1.192	2.145	3.040	5.052
		(AC-FT)	3.	5.	8.	13.

CUMULATIVE AREA = 0.05 SQ MI

180 KK *****
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 * E *
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181 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

208 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.04 SUBBASIN AREA

PRECIPITATION DATA

183 PB STORM 7.46 BASIN TOTAL PRECIPITATION

209 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.06 UNIFORM LOSS RATE
 RTIMP 60.77 PERCENT IMPERVIOUS AREA

208 UI INPUT UNITGRAPH, 93 ORDINATES, VOLUME = 1.00

3.6	8.3	13.5	18.0	24.8	31.4	38.9	47.3	56.5	67.1
75.9	77.9	77.6	75.1	67.2	59.1	51.6	47.0	42.2	38.2
34.9	31.6	29.0	26.5	24.2	22.4	20.7	19.1	17.8	16.6
15.4	14.4	13.5	12.8	11.8	11.2	10.5	9.9	9.6	9.1
8.7	8.3	7.9	7.5	7.2	6.9	6.5	6.3	6.0	5.7
5.5	5.2	4.9	4.8	4.5	4.3	4.1	3.9	3.8	3.6
3.4	3.3	3.1	3.0	2.9	2.7	2.6	2.5	2.3	2.2

2.9	2.8	2.7	2.6	2.5	2.4	2.3	2.2	2.1	2.0
2.0	1.9	1.8	1.7	1.6	1.5	1.5	1.4	1.4	1.3
1.3	1.2	1.2	1.1	1.1	1.0	1.0	0.9		

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HYDROGRAPH AT STATION F

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.34, TOTAL EXCESS = 5.12

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	249.10-HR	
13.	153.00	6.	3.	1.	1.	
		(INCHES)	1.197	2.155	3.066	5.114
		(AC-FT)	3.	5.	7.	12.

CUMULATIVE AREA = 0.05 SQ MI

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267 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

294 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.06 SUBBASIN AREA

PRECIPITATION DATA

269 PB STORM 7.46 BASIN TOTAL PRECIPITATION

295 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 38.92 PERCENT IMPERVIOUS AREA

294 UI INPUT UNITGRAPH, 109 ORDINATES, VOLUME = 1.00

3.9	9.4	15.2	20.9	27.1	34.7	42.5	51.2	60.7	70.5
82.1	94.6	104.6	107.1	107.6	105.5	99.6	90.2	80.7	72.1
66.3	61.1	55.2	51.4	47.4	43.7	40.6	37.6	34.8	32.6
30.4	28.5	26.6	24.9	23.5	22.2	20.8	19.7	18.6	17.8
16.7	15.9	14.9	14.4	13.6	13.3	12.7	12.3	11.7	11.2
10.8	10.4	10.0	9.6	9.2	8.8	8.6	8.2	7.9	7.6
7.3	7.0	6.7	6.6	6.2	6.0	5.8	5.5	5.4	5.2
4.9	4.8	4.6	4.4	4.2	4.1	3.9	3.7	3.6	3.4
3.3	3.2	3.1	3.0	2.8	2.7	2.7	2.6	2.5	2.4
2.3	2.2	2.1	2.0	2.0	1.9	1.8	1.7	1.7	1.6
1.6	1.5	1.4	1.4	1.3	1.3	1.3	1.2	1.1	

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HYDROGRAPH AT STATION G

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.83, TOTAL EXCESS = 4.63

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	249.10-HR	
18.	153.02	8.	3.	2.	1.	
		(INCHES)	1.152	2.007	2.822	4.625
		(AC-FT)	4.	7.	9.	15.

CUMULATIVE AREA = 0.06 SQ MI

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308 KK * H *

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309 IN TIME DATA FOR INPUT TIME SERIES
 JKMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

336 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.11 SUBBASIN AREA

PRECIPITATION DATA

311 PB STORM 7.46 BASIN TOTAL PRECIPITATION

337 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 46.94 PERCENT IMPERVIOUS AREA

336 UI INPUT UNITGRAPH, 161 ORDINATES, VOLUME = 1.00

3.2	7.2	11.8	16.6	21.8	26.1	30.9	38.0	43.8	50.3
57.5	64.8	72.9	81.1	89.7	99.5	110.0	120.1	128.4	130.8
132.2	132.3	130.9	128.5	122.0	114.2	106.3	98.3	91.0	85.1
80.7	76.3	71.4	67.0	63.9	60.6	57.2	54.1	51.5	48.9
46.5	44.1	41.7	40.1	38.3	36.5	34.9	33.3	31.9	30.5
29.3	28.2	27.1	25.9	24.9	24.0	23.1	22.3	21.6	20.7
19.9	19.4	18.5	18.0	17.4	16.8	16.5	16.0	15.6	15.3
14.8	14.4	13.9	13.6	13.3	12.9	12.5	12.2	11.9	11.6
11.3	10.9	10.7	10.4	10.1	9.9	9.6	9.4	9.1	8.8
8.6	8.4	8.1	8.1	7.8	7.5	7.3	7.1	6.9	6.8
6.6	6.5	6.3	6.1	6.0	5.8	5.7	5.5	5.4	5.2
5.1	4.9	4.8	4.7	4.5	4.4	4.3	4.2	4.1	4.0
3.9	3.8	3.7	3.6	3.5	3.4	3.3	3.2	3.1	3.1
3.0	3.0	2.9	2.8	2.7	2.7	2.6	2.5	2.4	2.4
2.3	2.2	2.2	2.1	2.1	2.0	2.0	1.9	1.9	1.8
1.8	1.7	1.7	1.6	1.6	1.6	1.5	1.5	1.5	1.4
1.4									

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HYDROGRAPH AT STATION H

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.44, TOTAL EXCESS = 5.02

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	249.10-HR
31.1	153.05	14.	6.	3.	1.
		1.177 (INCHES)	2.128	3.019	5.015
		7. (AC-FT)	13.	18.	30.

CUMULATIVE AREA = 0.11 SQ MI

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357 IN TIME DATA FOR INPUT TIME SERIES
 JKMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

384 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.08 SUBBASIN AREA

PRECIPITATION DATA

359 PB STORM 7.46 BASIN TOTAL PRECIPITATION

385 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 46.33 PERCENT IMPERVIOUS AREA

384 UI INPUT UNITGRAPH, 186 ORDINATES, VOLUME = 1.00

1.8	4.0	6.5	9.2	12.0	15.0	17.4	20.0	24.1	27.6
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+ 26. 153.02 (CFS) 12. 5. 3. 1.
 (INCHES) 1.319 2.580 3.740 6.439
 (AC-FT) 6. 12. 17. 29.
 CUMULATIVE AREA = 0.09 SQ MI

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 449 KK * OFF-N *
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Z5

450 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

477 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.84 SUBBASIN AREA

PRECIPITATION DATA

452 PB STORM 7.46 BASIN TOTAL PRECIPITATION

UNIFORM LOSS RATE

478 LU STRTL 0.20 INITIAL LOES
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIME 2.00 PERCENT IMPERVIOUS AREA

477 UI INPUT UNITGRAPH, 472 ORDINATES, VOLUME = 1.00

2.8	5.6	8.4	11.3	15.4	19.4	23.5	27.5	31.8	36.0
40.2	44.5	49.1	53.8	58.4	62.7	66.2	69.6	73.1	77.5
83.9	90.2	96.5	102.3	107.4	112.5	117.5	123.2	129.4	135.7
142.0	148.4	155.0	161.6	168.1	175.1	182.3	189.6	196.8	204.2
211.5	218.9	226.2	234.6	243.3	252.1	260.8	270.1	279.5	288.9
298.3	307.0	315.6	324.2	332.7	335.3	337.4	339.4	341.5	342.7
343.8	344.9	346.0	344.8	343.7	342.5	341.3	339.2	337.1	335.1
332.1	325.3	318.5	311.7	304.8	297.8	290.7	283.7	276.6	269.5
262.4	255.2	248.4	242.2	236.0	229.7	224.5	220.6	216.7	212.8
208.9	205.0	201.1	197.3	193.0	188.4	183.9	179.3	175.9	173.0
170.2	167.4	164.5	161.6	158.6	155.7	152.6	149.6	146.6	143.5
141.1	138.9	136.6	134.3	131.9	129.6	127.3	125.0	122.9	120.8
118.8	116.7	114.6	112.5	110.3	108.3	106.9	105.4	104.0	102.5
100.8	99.0	97.3	95.7	94.3	93.0	91.6	90.2	88.8	87.4
86.0	84.6	83.4	82.1	80.8	79.6	78.6	77.5	76.5	75.5
74.6	73.6	72.7	71.7	70.6	69.5	68.4	67.4	66.6	65.7
64.8	64.0	63.2	62.4	61.6	60.8	60.0	59.2	58.4	57.8
57.3	56.7	56.1	55.3	54.4	53.5	52.6	52.1	51.6	51.2
50.7	49.9	49.1	48.3	47.5	47.2	46.8	46.5	46.1	45.5
44.9	44.2	43.7	43.5	43.2	42.9	42.6	42.2	41.7	41.3
40.9	40.6	40.3	40.0	39.6	39.2	38.8	38.4	38.0	37.6
37.3	36.8	36.5	36.1	35.8	35.4	35.1	34.9	34.6	34.3
34.0	33.7	33.3	33.0	32.6	32.3	32.0	31.7	31.5	31.3
31.0	30.8	30.5	30.2	29.9	29.6	29.3	29.0	28.7	28.4
28.2	28.0	27.8	27.6	27.4	27.0	26.7	26.4	26.3	26.1
25.9	25.7	25.4	25.1	24.8	24.5	24.4	24.3	24.1	23.9
23.6	23.3	23.0	22.8	22.6	22.4	22.2	22.0	21.8	21.6
21.4	21.2	21.1	21.1	21.0	20.8	20.5	20.2	20.0	19.8
19.6	19.4	19.3	19.1	18.9	18.7	18.5	18.4	18.2	18.0
17.8	17.7	17.6	17.4	17.3	17.2	17.0	16.9	16.8	16.6
16.4	16.3	16.1	15.9	15.8	15.7	15.6	15.4	15.3	15.2
15.0	14.9	14.8	14.6	14.5	14.4	14.2	14.1	14.0	13.8
13.7	13.6	13.4	13.3	13.2	13.0	12.9	12.9	12.8	12.7
12.5	12.4	12.2	12.0	11.9	11.8	11.7	11.6	11.5	11.4
11.2	11.1	11.0	10.9	10.8	10.7	10.7	10.6	10.5	10.4
10.3	10.2	10.1	10.0	9.9	9.9	9.8	9.7	9.6	9.5
9.4	9.3	9.3	9.2	9.1	9.0	8.9	8.8	8.7	8.6
8.6	8.6	8.5	8.5	8.4	8.4	8.3	8.3	8.2	8.1
8.0	7.9	7.9	7.8	7.8	7.7	7.7	7.6	7.5	7.4
7.3	7.2	7.1	7.1	7.0	7.0	6.9	6.9	6.8	6.8
6.8	6.7	6.6	6.5	6.4	6.4	6.3	6.3	6.2	6.2
6.1	6.0	5.9	5.8	5.8	5.8	5.7	5.7	5.6	5.6
5.5	5.5	5.4	5.4	5.4	5.3	5.3	5.2	5.2	5.1
5.1	5.1	5.0	5.0	4.9	4.9	4.8	4.8	4.7	4.7
4.7	4.6	4.6	4.5	4.5	4.4	4.4	4.3	4.3	4.3
4.2	4.2	4.2	4.2	4.2	4.2	4.1	4.1	4.0	4.0
4.0	3.9	3.9	3.8	3.8	3.7	3.7	3.6	3.6	3.6
3.6	3.6								

 HYDROGRAPH AT STATION OFF-N

TOTAL RAINFALL = 7.46, TOTAL LOSS = 4.55, TOTAL EXCESS = 2.92

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	249.10-HR
162.	153.62	86.	34.	15.	6.	
		(INCHES)	0.945	1.484	1.969	2.916
		(AC-FT)	43.	67.	89.	131.

CUMULATIVE AREA = 0.84 SQ MI

528 KK * DUMMY2 *

530 HC HYDROGRAPH COMBINATION
ICOMP 6 NUMBER OF HYDROGRAPHS TO COMBINE

HYDROGRAPH AT STATION DUMMY2

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	249.10-HR
252.	153.17	134.	56.	26.	12.	
		(INCHES)	1.010	1.703	2.344	3.663
		(AC-FT)	66.	112.	154.	241.

CUMULATIVE AREA = 1.23 SQ MI

532 KK * K *

533 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 60 TIME INTERVAL IN MINUTES
JXDATE 31DEC99 STARTING DATE
JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

560 BA SUBBASIN CHARACTERISTICS
TAREA, 0.06 SUBBASIN AREA

PRECIPITATION DATA

535 PB STORM 7.46 BASIN TOTAL PRECIPITATION

561 LU UNIFORM LOSS RATE
STRTL 0.20 INITIAL LOSS
CNSTL 0.06 UNIFORM LOSS RATE
RTIMP 47.94 PERCENT IMPERVIOUS AREA

560 UI INPUT UNITGRAPH, 151 ORDINATES, VOLUME = 1.00

2.0	4.7	7.7	10.8	14.1	16.7	20.6	24.8	28.5	33.1
37.8	42.8	48.1	53.5	59.8	66.5	72.9	77.8	79.3	80.1
79.7	78.5	76.3	71.3	66.3	61.2	56.3	52.0	49.2	46.4
43.3	40.5	38.4	36.3	34.1	32.3	30.6	28.9	27.4	25.9
24.6	23.5	22.3	21.3	20.3	19.3	18.4	17.7	17.0	16.3
15.5	14.9	14.3	13.8	13.3	12.8	12.2	11.8	11.3	10.9
10.6	10.2	10.0	9.7	9.4	9.2	8.9	8.6	8.3	8.1
7.9	7.7	7.4	7.3	7.1	6.8	6.6	6.5	6.3	6.1
6.0	5.8	5.6	5.5	5.3	5.1	5.0	4.9	4.8	4.6
4.5	4.3	4.2	4.1	4.0	3.9	3.8	3.7	3.6	3.5
3.4	3.3	3.2	3.1	3.0	2.9	2.8	2.7	2.7	2.6
2.5	2.4	2.4	2.3	2.3	2.2	2.1	2.1	2.0	2.0
1.9	1.9	1.8	1.8	1.7	1.7	1.6	1.6	1.6	1.5
1.5	1.4	1.4	1.3	1.3	1.3	1.2	1.2	1.2	1.1
1.1	1.1	1.0	1.0	1.0	1.0	1.0	0.9	0.9	0.9
0.8									


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HYDROGRAPH AT STATION K
TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.31, TOTAL EXCESS = 5.16
PEAK FLOW      TIME
(CFS)         (HR)
+ 18.         153.05
              (CFS)
              (INCHES)
              (AC-FT)
              6-HR      24-HR      72-HR      249.10-HR
              8.         4.         2.         1.
              1.196     2.184     3.094     5.149
              4.         7.         10.        17.
CUMULATIVE AREA = 0.06 SQ MI

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579 KK * L *
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580 IN      TIME DATA FOR INPUT TIME SERIES
           JKMIN      60  TIME INTERVAL IN MINUTES
           JKDATE     31DEC99  STARTING DATE
           JYTIME     2400  STARTING TIME

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SUBBASIN RUNOFF DATA

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607 BA      SUBBASIN CHARACTERISTICS
           TAREA,      0.03  SUBBASIN AREA

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PRECIPITATION DATA

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582 PB      STORM      7.46  BASIN TOTAL PRECIPITATION

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608 LU      UNIFORM LOSS RATE
           STRTL      0.20  INITIAL LOSS
           CNSTL      0.06  UNIFORM LOSS RATE
           RTIME      70.00  PERCENT IMPERVIOUS AREA

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607 UI      INPUT UNITGRAPH, 67 ORDINATES, VOLUME = 1.00
           6.4      14.7      22.9      33.6      45.9      59.7      76.2      91.6      94.5      91.6
           79.6      66.3      57.2      49.1      43.4      38.0      33.6      29.7      26.7      23.9
           21.5      19.6      17.7      16.2      14.9      13.7      12.6      11.7      11.0      10.3
           9.6       9.0       8.5       7.9       7.5       7.0       6.6       6.1       5.8       5.4
           5.1       4.8       4.5       4.2       4.0       3.7       3.5       3.2       3.0       2.8
           2.7       2.5       2.4       2.3       2.1       2.0       1.9       1.7       1.6       1.5
           1.4       1.4       1.3       1.2       1.1       1.1       1.0

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HYDROGRAPH AT STATION L
TOTAL RAINFALL = 7.46, TOTAL LOSS = 1.29, TOTAL EXCESS = 6.18
PEAK FLOW      TIME
(CFS)         (HR)
+ 10.         153.00
              (CFS)
              (INCHES)
              (AC-FT)
              6-HR      24-HR      72-HR      249.10-HR
              5.         2.         1.         1.
              1.303     2.494     3.603     6.168
              2.         4.         6.         11.
CUMULATIVE AREA = 0.03 SQ MI

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617 KK * M *
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618 IN      TIME DATA FOR INPUT TIME SERIES
           JKMIN      60  TIME INTERVAL IN MINUTES
           JKDATE     31DEC99  STARTING DATE

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JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

645 BA SUBBASIN CHARACTERISTICS
TAREA, 0.07 SUBBASIN AREA

PRECIPITATION DATA

620 PB STORM 7.46 BASIN TOTAL PRECIPITATION

UNIFORM LOSS RATE

STRTL 0.20 INITIAL LOSS
CNSTL 0.04 UNIFORM LOSS RATE
RTIMP 95.00 PERCENT IMPERVIOUS AREA

645 UI INPUT UNITGRAPH, 160 ORDINATES, VOLUME = 1.00

1.9	4.4	7.2	10.0	13.2	15.8	18.8	23.1	26.6	30.5
34.9	39.3	44.2	49.2	54.4	60.4	66.8	72.8	77.6	79.0
79.8	79.7	78.8	77.4	73.1	68.4	63.6	58.8	54.4	51.0
48.4	45.7	42.7	40.2	38.3	36.3	34.2	32.4	30.8	29.2
27.8	26.4	25.0	24.0	22.9	21.8	20.9	19.9	19.0	18.2
17.5	16.9	16.2	15.5	14.9	14.4	13.8	13.4	13.0	12.4
11.9	11.5	11.0	10.8	10.4	10.1	9.9	9.6	9.4	9.1
8.9	8.6	8.3	8.1	7.9	7.7	7.5	7.3	7.1	6.9
6.7	6.5	6.4	6.2	6.1	5.9	5.7	5.6	5.4	5.3
5.1	5.0	4.9	4.8	4.6	4.5	4.4	4.3	4.1	4.1
4.0	3.9	3.7	3.7	3.6	3.5	3.4	3.3	3.2	3.1
3.0	3.0	2.9	2.8	2.7	2.6	2.5	2.5	2.4	2.4
2.3	2.2	2.2	2.1	2.1	2.0	2.0	1.9	1.9	1.8
1.8	1.8	1.7	1.6	1.6	1.6	1.5	1.5	1.5	1.4
1.4	1.3	1.3	1.3	1.2	1.2	1.2	1.1	1.1	1.1
1.1	1.0	1.0	1.0	1.0	0.9	0.9	0.9	0.9	0.8

HYDROGRAPH AT STATION M

TOTAL RAINFALL = 7.46, TOTAL LOSS = 0.17, TOTAL EXCESS = 7.30

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	249.10-HR	
20.	153.05	10.	5.	2.	1.	
		(INCHES)	1.378	2.825	4.154	7.286
		(AC-FT)	5.	10.	15.	26.

CUMULATIVE AREA = 0.07 SQ MI

664 KK

* N *

665 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 60 TIME INTERVAL IN MINUTES
JXDATE 31DEC99 STARTING DATE
JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

692 BA SUBBASIN CHARACTERISTICS
TAREA, 0.04 SUBBASIN AREA

PRECIPITATION DATA

667 PB STORM 7.46 BASIN TOTAL PRECIPITATION

UNIFORM LOSS RATE

STRTL 0.20 INITIAL LOSS
CNSTL 0.07 UNIFORM LOSS RATE
RTIMP 43.11 PERCENT IMPERVIOUS AREA

692 UI INPUT UNITGRAPH, 117 ORDINATES, VOLUME = 1.00

2.0	4.9	8.0	11.2	13.9	18.3	22.0	26.6	31.3	36.5
42.0	48.4	55.0	59.8	61.1	61.4	60.4	57.7	52.8	47.8
42.9	39.2	36.4	33.4	30.7	28.6	26.5	24.6	23.0	21.4
19.9	18.7	17.5	16.5	15.5	14.5	13.7	13.1	12.3	11.6
11.1	10.5	10.1	9.5	9.1	8.5	8.3	7.8	7.6	7.3
7.1	6.8	6.5	6.3	6.1	5.8	5.6	5.5	5.2	5.0
4.9	4.7	4.5	4.4	4.2	4.0	3.9	3.8	3.7	3.5
3.4	3.2	3.1	3.0	2.9	2.8	2.7	2.6	2.5	2.4
2.4	2.3	2.2	2.1	2.0	1.9	1.9	1.8	1.7	1.7

1.6	1.6	1.5	1.5	1.4	1.4	1.3	1.3	1.2	1.2
1.1	1.1	1.0	1.0	1.0	1.0	0.9	0.9	0.9	0.8
0.8	0.8	0.7	0.7	0.7	0.7	0.6			

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HYDROGRAPH AT STATION N

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.60, TOTAL EXCESS = 4.86

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	249.10-HR
11.	153.02	5.	2.	1.	0.
		(INCHES) 1.173	2.083	2.941	4.856
		(AC-FT) 2.	4.	6.	10.

CUMULATIVE AREA = 0.04 SQ MI

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708 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 31DEC99 STARTING DATE
 JXTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

735 BA SUBBASIN CHARACTERISTICS
 TAREA, 0.04 SUBBASIN AREA

PRECIPITATION DATA

710 PB STORM 7.46 BASIN TOTAL PRECIPITATION

736 LU UNIFORM LOSS RATE
 STRTL 0.20 INITIAL LOSS
 CNSTL 0.07 UNIFORM LOSS RATE
 RTIMP 50.00 PERCENT IMPERVIOUS AREA

735 UI INPUT UNITGRAPH, 93 ORDINATES, VOLUME = 1.00

3.7	8.6	14.0	18.7	25.8	32.7	40.4	49.1	58.6	69.6
79.8	82.0	82.1	79.9	72.3	63.8	55.8	50.5	45.7	41.1
37.6	34.0	31.3	28.6	26.1	24.3	22.3	20.7	19.1	17.9
16.7	15.5	14.6	13.7	12.8	12.1	11.3	10.7	10.3	9.8
9.4	8.9	8.5	8.1	7.7	7.4	7.1	6.7	6.5	6.2
5.9	5.6	5.3	5.1	4.9	4.7	4.5	4.3	4.1	3.9
3.7	3.6	3.4	3.3	3.1	3.0	2.8	2.7	2.6	2.5
2.3	2.2	2.1	2.0	2.0	1.9	1.8	1.7	1.7	1.6
1.5	1.4	1.4	1.3	1.3	1.2	1.2	1.1	1.0	1.0
1.0	0.9	0.9							

*** *** *** *** ***

HYDROGRAPH AT STATION O

TOTAL RAINFALL = 7.46, TOTAL LOSS = 2.32, TOTAL EXCESS = 5.15

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	249.10-HR
12.	153.00	5.	2.	1.	1.
		(INCHES) 1.199	2.162	3.076	5.135
		(AC-FT) 3.	5.	7.	11.

CUMULATIVE AREA = 0.04 SQ MI

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749 IN TIME DATA FOR INPUT TIME SERIES
JKMIN 60 TIME INTERVAL IN MINUTES
JKDATE 31DEC99 STARTING DATE
JKTIME 2400 STARTING TIME

SUBBASIN RUNOFF DATA

776 BA SUBBASIN CHARACTERISTICS
TAREA, 0.03 SUBBASIN AREA

PRECIPITATION DATA

751 PB STORM 7.46 BASIN TOTAL PRECIPITATION

777 LU UNIFORM LOSS RATE
STRTL 0.20 INITIAL LOSS
CNSTL 0.06 UNIFORM LOSS RATE
RTIMP 56.38 PERCENT IMPERVIOUS AREA

776 UI INPUT UNITGRAPH, 102 ORDINATES, VOLUME = 1.00

2.6	6.2	10.0	13.4	18.2	22.8	28.3	34.2	40.5	47.7
55.7	62.7	64.3	64.5	63.1	58.5	52.4	46.3	41.5	38.1
34.4	31.5	29.0	26.5	24.5	22.6	20.8	19.4	17.9	16.7
15.6	14.5	13.7	12.8	12.0	11.3	10.7	10.1	9.6	8.9
8.6	8.1	7.8	7.5	7.2	6.9	6.6	6.3	6.0	5.8
5.6	5.3	5.1	4.9	4.7	4.5	4.3	4.1	4.0	3.8
3.6	3.5	3.3	3.2	3.1	3.0	2.8	2.7	2.6	2.5
2.4	2.3	2.2	2.1	2.0	1.9	1.8	1.8	1.7	1.6
1.6	1.5	1.5	1.4	1.3	1.3	1.2	1.2	1.1	1.1
1.0	1.0	1.0	0.9	0.9	0.8	0.8	0.8	0.8	0.7
0.7	0.7								

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HYDROGRAPH AT STATION P

TOTAL RAINFALL = 7.46, TOTAL LOSS = 1.90, TOTAL EXCESS = 5.56

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	249.10-HR
10.	153.02	5.	2.	1.	0.
		(INCHES) 1.245	2.313	3.303	5.562
		(AC-FT) 2.	4.	6.	10.

CUMULATIVE AREA = 0.03 SQ MI

790 KE *****
* DUMMY3 *

792 HC HYDROGRAPH COMBINATION
ICOMP 6 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

HYDROGRAPH AT STATION DUMMY3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	249.10-HR
81.	153.02	37.	18.	8.	4.
		(INCHES) 1.255	2.376	3.415	5.799
		(AC-FT) 18.	35.	50.	85.

CUMULATIVE AREA = 0.27 SQ MI

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+									

+	HYDROGRAPH AT	A	33.	153.07	16.	7.	4.	0.12
+	HYDROGRAPH AT	B	13.	153.00	6.	3.	1.	0.04
+	HYDROGRAPH AT	C	18.	153.02	8.	4.	2.	0.06
+	HYDROGRAPH AT	D	14.	153.02	6.	3.	1.	0.05
+	HYDROGRAPH AT	E	11.	153.00	5.	2.	1.	0.04
+	5 COMBINED AT	DUMMY1	89.	153.02	41.	19.	9.	0.31
+	HYDROGRAPH AT	F	13.	153.00	6.	3.	1.	0.05
+	HYDROGRAPH AT	G	18.	153.02	8.	3.	2.	0.06
+	HYDROGRAPH AT	H	31.	153.05	14.	6.	3.	0.11
+	HYDROGRAPH AT	I	23.	153.08	11.	5.	2.	0.08
+	HYDROGRAPH AT	J	26.	153.02	12.	6.	3.	0.09
+	HYDROGRAPH AT	OFF-N	162.	153.62	86.	34.	15.	0.84
+	6 COMBINED AT	DUMMY2	252.	153.17	134.	56.	26.	1.23
+	HYDROGRAPH AT	K	18.	153.05	8.	4.	2.	0.06
+	HYDROGRAPH AT	L	10.	153.00	5.	2.	1.	0.03
+	HYDROGRAPH AT	M	20.	153.05	10.	5.	2.	0.07
+	HYDROGRAPH AT	N	11.	153.02	5.	2.	1.	0.04
+	HYDROGRAPH AT	O	12.	153.00	5.	2.	1.	0.04
+	HYDROGRAPH AT	P	10.	153.02	5.	2.	1.	0.03
+	6 COMBINED AT	DUMMY3	81.	153.02	37.	18.	8.	0.27

*** NORMAL END OF BEC-1L ***



Ultimate Conditions HEC-RAS Model

25 100year 24hour Model

HEC-RAS Version 3.1.3 May 2005
 U.S. Army Corp of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

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X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X       X   X   X   X
X   X   X       X   X       X   X   X   X
XXXXXXXX XXXX   X   XXX XXXX XXXXXXX XXXX
X   X   X       X   X       X   X   X   X
X   X   X       X   X       X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
    
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PROJECT DATA
 Project Title: GREENBRIAR FARMS
 Project File : GB.prj
 Run Date and Time: 7/14/2005 3:37:39 PM

Project in English units

Project Description:

References:

- 1) Offsite Drainage Improvements-Metro Air Park, Exhibit D-2 (Offsite improved channels and Interstate 5 crossing)
- 2) 100yr-10day & 10yr-10day, STA 0+00, West Drain Stages, Mark Kubik @ Mead & Hunt Inc.

PLAN DATA

Plan Title: P3-100YR24HR-DEVELOP
 Plan File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.p03

Geometry Title: DEVELOP
 Geometry File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.g01

Flow Title :
 Flow File :

Plan Summary Information:

Number of: Cross Sections = 207 Multiple Openings = 0
 Culverts = 6 Inline Structures = 1
 Bridges = 0 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

Profile Output Table - Concise Table 1

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	E.G. Elev (ft)	Vel Chnl (ft/s)	Flow Area (sq ft)
WEST DETENTION	1	5540	Max WS	5.89	11.00	14.90	14.90	0.00	1472.20
WEST DETENTION	1	5460	Max WS	5.88	11.00	14.90	14.90	0.00	1351.94
WEST DETENTION	1	5438	Max WS	5.88	11.00	14.90	14.90	0.01	628.37
WEST DETENTION	1	5412	Max WS	5.88	11.00	14.90	14.90	0.01	531.02
WEST DETENTION	1	4950	Max WS	5.87	11.00	14.90	14.90	0.01	514.07
WEST DETENTION	1	4907	Max WS	5.87	11.00	14.90	14.90	0.01	671.81
WEST DETENTION	1	4611	Max WS	9.41	11.00	14.90	14.90	0.01	659.33
WEST DETENTION	1	4537	Max WS	11.83	11.00	14.90	14.90	0.02	514.64
WEST DETENTION	1	3571	Max WS	11.83	11.00	14.90	14.90	0.02	512.48
WEST DETENTION	1	3570							
WEST DETENTION	1	3417	Max WS	11.67	11.00	14.87	14.87	0.02	508.82
WEST DETENTION	1	3400	Max WS	11.67	11.00	14.87	14.87	0.02	508.48
WEST DETENTION	1	3383	Max WS	13.96	11.00	14.87	14.87	0.03	509.83
WEST DETENTION	1	2760	Max WS	16.03	11.00	14.87	14.87	0.03	507.98
WEST DETENTION	1	2722	Max WS	18.58	11.00	14.87	14.87	0.02	909.49
WEST DETENTION	1	2635	Max WS	18.58	11.00	14.87	14.87	0.02	916.87
WEST DETENTION	1	2586	Max WS	18.58	11.00	14.87	14.87	0.01	1301.49
WEST DETENTION	1	2545	Max WS	18.57	11.00	14.87	14.87	0.01	1301.91

WEST DETENTION	1	2507	Max WS	18.57	11.00	14.87	14.87	0.02	925.62
WEST DETENTION	1	2402	Max WS	18.57	11.00	14.87	14.87	0.02	914.81
WEST DETENTION	1	2367	Max WS	21.74	11.00	14.87	14.87	0.04	519.80
WEST DETENTION	1	1987	Max WS	21.73	11.00	14.87	14.87	0.04	510.69
WEST DETENTION	1	1986		Culvert					
WEST DETENTION	1	1922	Max WS	21.70	11.00	14.79	14.79	0.04	497.47
WEST DETENTION	1	1891	Max WS	21.70	11.00	14.79	14.79	0.04	496.93
WEST DETENTION	1	1815	Max WS	23.76	11.00	14.79	14.79	0.02	1052.61
SOUTH DETENTION	3	1636	Max WS	49.37	11.00	14.79	14.79	0.03	1451.36
SOUTH DETENTION	3	1561	Max WS	49.37	11.00	14.79	14.79	0.05	991.54
SOUTH DETENTION	3	1153	Max WS	55.95	11.00	14.79	14.79	0.05	1212.08
SOUTH DETENTION	3	1080	Max WS	55.95	11.00	14.79	14.79	0.05	1045.94
SOUTH DETENTION	3	1012	Max WS	61.46	11.00	14.79	14.79	0.04	1598.14
SOUTH DETENTION	3	945	Max WS	61.46	11.00	14.79	14.79	0.05	1123.51
SOUTH DETENTION	3	942		Inl Struct					
SOUTH DETENTION	3	940	Max WS	61.46	2.70	14.79	14.79	0.02	4385.12
SOUTH DETENTION	3	930	Max WS	61.46	2.70	14.79	14.79	0.02	4385.12
SOUTH DETENTION	3	500		Culvert					
SOUTH DETENTION	3	150	Max WS	61.43	2.20	8.84	8.84	0.03	1919.15
SOUTH DETENTION	3	100	Max WS	61.25	2.20	8.84	8.84	0.03	1919.15
SOUTH DETENTION	3	90	Max WS	61.22	2.20	8.84	8.84	0.03	1919.15
OFFSITE	RR NORTH	5350	Max WS	402.66	9.00	15.70	15.82	2.72	147.89
OFFSITE	RR NORTH	1100	Max WS	383.17	6.00	8.84	9.82	7.97	48.09
OFFSITE	RR NORTH	1050	Max WS	386.23	2.30	9.03	9.08	1.66	232.12
OFFSITE	RR NORTH	100	Max WS	384.32	2.20	8.84	8.88	1.70	226.60
OFFSITE	RR SOUTH	3500	Max WS	445.54	2.20	8.84	8.90	1.97	226.60
OFFSITE	RR SOUTH	3450	Max WS	445.51	2.20	8.85	8.89	1.52	293.71
OFFSITE	RR SOUTH	3300		Culvert					
OFFSITE	RR SOUTH	3100	Max WS	444.62	2.00	8.46	8.49	1.58	281.07
OFFSITE	RR SOUTH	100	Max WS	181.03	1.80	7.56	7.58	0.98	184.16
EAST DETENTION	2	5600	Max WS	-0.89	11.00	14.90	14.90	0.00	1472.54
EAST DETENTION	2	5560	Max WS	-0.89	11.00	14.90	14.90	0.00	1464.29
EAST DETENTION	2	5520	Max WS	-0.89	11.00	14.90	14.90	0.00	1362.87
EAST DETENTION	2	5480	Max WS	-0.89	11.00	14.90	14.90	0.00	624.64
EAST DETENTION	2	5455	Max WS	-0.89	11.00	14.90	14.90	0.00	514.64
EAST DETENTION	2	4920	Max WS	-0.91	11.00	14.90	14.90	0.00	513.99
EAST DETENTION	2	4880	Max WS	-0.91	11.00	14.90	14.90	0.00	898.34
EAST DETENTION	2	4850	Max WS	-0.92	11.00	14.90	14.90	0.00	2129.66
EAST DETENTION	2	4775	Max WS	6.92	11.00	14.90	14.90	0.01	965.98
EAST DETENTION	2	4653	Max WS	8.67	11.00	14.90	14.90	0.02	553.84
EAST DETENTION	2	4126	Max WS	8.66	11.00	14.90	14.90	0.02	515.17
EAST DETENTION	2	3563	Max WS	12.39	11.00	14.90	14.90	0.02	513.52
EAST DETENTION	2	3562		Culvert					
EAST DETENTION	2	3414	Max WS	11.88	11.00	14.86	14.86	0.02	507.39
EAST DETENTION	2	3335	Max WS	11.88	11.00	14.86	14.86	0.03	464.78
EAST DETENTION	2	3282	Max WS	14.06	11.00	14.86	14.86	0.06	221.68
EAST DETENTION	2	2946	Max WS	20.50	11.00	14.86	14.86	0.10	195.65
EAST DETENTION	2	2945		Culvert					
EAST DETENTION	2	2855	Max WS	20.38	11.00	14.79	14.79	0.09	229.22
EAST DETENTION	2	2804	Max WS	20.38	11.00	14.79	14.79	0.08	254.88
EAST DETENTION	2	1864	Max WS	25.61	11.00	14.79	14.79	0.03	809.85
EAST DETENTION	2	1800	Max WS	25.61	11.00	14.79	14.79	0.02	1318.29
DUMMY	4	1	Max WS	5.00	11.00	14.90	14.90	0.01	337.64
DUMMY	4	0	Max WS	5.00	11.00	14.90	14.90	0.01	339.48

Profile Output Table - ChannelVolume

River	Reach	River Sta	Profile	Volume (acre-ft)
WEST DETENTION	1	5540	Max WS	57.07
WEST DETENTION	1	5460	Max WS	54.64
WEST DETENTION	1	5438	Max WS	52.39
WEST DETENTION	1	5412	Max WS	52.01
WEST DETENTION	1	4950	Max WS	46.45
WEST DETENTION	1	4907	Max WS	45.70
WEST DETENTION	1	4611	Max WS	41.03
WEST DETENTION	1	4537	Max WS	40.07
WEST DETENTION	1	3571	Max WS	28.69
WEST DETENTION	1	3570		Culvert
WEST DETENTION	1	3417	Max WS	26.86
WEST DETENTION	1	3400	Max WS	26.66
WEST DETENTION	1	3383	Max WS	26.48
WEST DETENTION	1	2760	Max WS	19.28
WEST DETENTION	1	2722	Max WS	18.30
WEST DETENTION	1	2635	Max WS	16.47
WEST DETENTION	1	2586	Max WS	14.58
WEST DETENTION	1	2545	Max WS	13.37
WEST DETENTION	1	2507	Max WS	11.73
WEST DETENTION	1	2402	Max WS	9.41
WEST DETENTION	1	2367	Max WS	8.39
WEST DETENTION	1	1987	Max WS	3.89
WEST DETENTION	1	1986		Culvert
WEST DETENTION	1	1922	Max WS	3.15
WEST DETENTION	1	1891	Max WS	2.79
WEST DETENTION	1	1815	Max WS	1.44
SOUTH DETENTION	3	1636	Max WS	80.12
SOUTH DETENTION	3	1561	Max WS	78.00
SOUTH DETENTION	3	1153	Max WS	67.68
SOUTH DETENTION	3	1080	Max WS	65.79
SOUTH DETENTION	3	1012	Max WS	63.73
SOUTH DETENTION	3	945	Max WS	61.64
SOUTH DETENTION	3	942		Inl Struct
SOUTH DETENTION	3	940	Max WS	61.32

SOUTH DETENTION	3	930	Max WS	60.32
SOUTH DETENTION	3	500		Culvert
SOUTH DETENTION	3	150	Max WS	3.87
SOUTH DETENTION	3	100	Max WS	1.67
SOUTH DETENTION	3	90	Max WS	1.23
OFFSITE	R8 NORTH	5350	Max WS	17.46
OFFSITE	R8 NORTH	1100	Max WS	5.42
OFFSITE	R8 NORTH	1050	Max WS	5.26
OFFSITE	R8 NORTH	100	Max WS	0.26
OFFSITE	R8 SOUTH	3500	Max WS	18.78
OFFSITE	R8 SOUTH	3450	Max WS	18.48
OFFSITE	R8 SOUTH	3300		Culvert
OFFSITE	R8 SOUTH	3100	Max WS	16.17
OFFSITE	R8 SOUTH	100	Max WS	
EAST DETENTION	2	5600	Max WS	60.85
EAST DETENTION	2	5560	Max WS	59.64
EAST DETENTION	2	5520	Max WS	58.23
EAST DETENTION	2	5480	Max WS	55.94
EAST DETENTION	2	5455	Max WS	55.49
EAST DETENTION	2	4920	Max WS	49.17
EAST DETENTION	2	4880	Max WS	48.19
EAST DETENTION	2	4850	Max WS	42.39
EAST DETENTION	2	4775	Max WS	35.26
EAST DETENTION	2	4653	Max WS	33.14
EAST DETENTION	2	4126	Max WS	26.66
EAST DETENTION	2	3563	Max WS	20.02
EAST DETENTION	2	3562		Culvert
EAST DETENTION	2	3414	Max WS	18.27
EAST DETENTION	2	3335	Max WS	17.39
EAST DETENTION	2	3282	Max WS	16.97
EAST DETENTION	2	2946	Max WS	15.36
EAST DETENTION	2	2945		Culvert
EAST DETENTION	2	2855	Max WS	14.92
EAST DETENTION	2	2804	Max WS	14.63
EAST DETENTION	2	1864	Max WS	3.15
EAST DETENTION	2	1800	Max WS	1.59
DUMMY	4	1	Max WS	0.10
DUMMY	4	0	Max WS	0.02

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100year 10day Model

HEC-RAS Version 3.1.3 May 2005
 U.S. Army Corp of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

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X   X XXXXXX XXXX   XXXX   XX   XXXX
X   X X   X   X X   X X X   X X X
X   X X   X   X   X X X   X X X
XXXXXXXX XXXX X   XXX XXXX XXXXXX XXXX
X   X X   X   X   X X X   X X   X
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X   X XXXXXX XXXX   X X X   X XXXX
    
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PROJECT DATA

Project Title: GREENBRIAR FARMS
 Project File : GB.prj
 Run Date and Time: 7/14/2005 9:49:06 AM

Project in English units

Project Description:

References:

- 1) Offsite Drainage Improvements-Metro Air Park, Exhibit D-2 (Offsite improved channels and Interstate 5 crossing)
- 2) 100yr-10day & 10yr-10day, STA 0+00, West Drain Stages, Mark Kubik @ Mead & Hunt Inc.

PLAN DATA

Plan Title: P1-100YR10DAY-DEVELOP
 Plan File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.p01

Geometry Title: DEVELOP
 Geometry File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.g01

Flow Title :
 Flow File :

Plan Summary Information:

Number of: Cross Sections = 207 Multiple Openings = 0
 Culverts = 6 Inline Structures = 1
 Bridges = 0 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

Profile Output Table - Concise Table 1

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	E.G. Elev (ft)	Vel Chnl (ft/s)	Flow Area (sq ft)
WEST DETENTION	1	5540	Max WS	6.79	11.00	14.87	14.87	0.00	1461.31
WEST DETENTION	1	5460	Max WS	6.77	11.00	14.87	14.87	0.01	1341.81
WEST DETENTION	1	5438	Max WS	6.76	11.00	14.87	14.87	0.01	623.47
WEST DETENTION	1	5412	Max WS	6.75	11.00	14.87	14.87	0.01	526.81
WEST DETENTION	1	4950	Max WS	6.72	11.00	14.87	14.87	0.01	510.06
WEST DETENTION	1	4907	Max WS	6.71	11.00	14.87	14.87	0.01	666.62
WEST DETENTION	1	4611	Max WS	9.19	11.00	14.87	14.87	0.01	654.24
WEST DETENTION	1	4537	Max WS	10.90	11.00	14.87	14.87	0.02	510.62
WEST DETENTION	1	3571	Max WS	10.81	11.00	14.87	14.87	0.02	508.48
WEST DETENTION	1	3570	Culvert						
WEST DETENTION	1	3417	Max WS	9.75	11.00	14.84	14.84	0.02	505.40
WEST DETENTION	1	3400	Max WS	9.75	11.00	14.84	14.84	0.02	505.06
WEST DETENTION	1	3383	Max WS	11.25	11.00	14.84	14.84	0.02	506.40
WEST DETENTION	1	2760	Max WS	12.95	11.00	14.84	14.84	0.03	504.55
WEST DETENTION	1	2722	Max WS	14.85	11.00	14.84	14.84	0.02	903.57
WEST DETENTION	1	2635	Max WS	14.85	11.00	14.84	14.84	0.02	910.91
WEST DETENTION	1	2586	Max WS	14.84	11.00	14.84	14.84	0.01	1293.13
WEST DETENTION	1	2545	Max WS	14.83	11.00	14.84	14.84	0.01	1293.55
WEST DETENTION	1	2507	Max WS	14.83	11.00	14.84	14.84	0.02	919.60

WEST DETENTION	1	2402	Max WS	14.82	11.00	14.84	14.84	0.02	908.85
WEST DETENTION	1	2367	Max WS	17.26	11.00	14.84	14.84	0.03	516.29
WEST DETENTION	1	1987	Max WS	17.24	11.00	14.84	14.84	0.03	507.23
WEST DETENTION	1	1986		Culvert					
WEST DETENTION	1	1922	Max WS	15.26	11.00	14.80	14.80	0.03	498.95
WEST DETENTION	1	1891	Max WS	15.26	11.00	14.80	14.80	0.03	498.42
WEST DETENTION	1	1815	Max WS	17.30	11.00	14.80	14.80	0.02	1055.66
SOUTH DETENTION	3	1636	Max WS	42.53	11.00	14.80	14.80	0.03	1455.50
SOUTH DETENTION	3	1561	Max WS	42.53	11.00	14.80	14.80	0.04	994.38
SOUTH DETENTION	3	1153	Max WS	53.13	11.00	14.80	14.80	0.04	1215.51
SOUTH DETENTION	3	1080	Max WS	53.12	11.00	14.80	14.80	0.05	1048.91
SOUTH DETENTION	3	1012	Max WS	61.47	11.00	14.80	14.80	0.04	1602.65
SOUTH DETENTION	3	945	Max WS	61.46	11.00	14.80	14.80	0.05	1126.79
SOUTH DETENTION	3	942		Inl Struct					
SOUTH DETENTION	3	940	Max WS	61.46	2.70	14.80	14.80	0.02	4389.69
SOUTH DETENTION	3	930	Max WS	61.46	2.70	14.80	14.80	0.02	4389.69
SOUTH DETENTION	3	500		Culvert					
SOUTH DETENTION	3	150	Max WS	46.18	2.20	11.21	11.21	0.02	2906.49
SOUTH DETENTION	3	100	Max WS	46.18	2.20	11.21	11.21	0.02	2906.49
SOUTH DETENTION	3	90	Max WS	46.18	2.20	11.21	11.21	0.02	2906.49
OFFSITE	R8 NORTH	5350	Max WS	251.59	9.00	14.18	14.27	2.46	102.28
OFFSITE	R8 NORTH	1100	Max WS	19.48	6.00	11.21	11.21	0.18	110.05
OFFSITE	R8 NORTH	1050	Max WS	19.48	2.30	11.21	11.21	0.06	345.95
OFFSITE	R8 NORTH	100	Max WS	19.46	2.20	11.21	11.21	0.06	350.01
OFFSITE	R8 SOUTH	3500	Max WS	65.63	2.20	11.21	11.21	0.19	350.01
OFFSITE	R8 SOUTH	3450	Max WS	65.63	2.20	11.21	11.21	0.15	440.12
OFFSITE	R8 SOUTH	3300		Culvert					
OFFSITE	R8 SOUTH	3100	Max WS	65.63	2.00	11.20	11.20	0.15	449.24
OFFSITE	R8 SOUTH	100	Max WS	63.33	1.80	11.19	11.19	0.17	365.14
EAST DETENTION	2	5600	Max WS	-1.79	11.00	14.87	14.87	0.00	1461.65
EAST DETENTION	2	5560	Max WS	-1.80	11.00	14.87	14.87	0.00	1453.45
EAST DETENTION	2	5520	Max WS	-1.80	11.00	14.87	14.87	0.00	1352.68
EAST DETENTION	2	5460	Max WS	-1.82	11.00	14.87	14.87	0.00	619.76
EAST DETENTION	2	5455	Max WS	-1.82	11.00	14.87	14.87	0.00	510.62
EAST DETENTION	2	4920	Max WS	-1.87	11.00	14.87	14.87	0.00	509.97
EAST DETENTION	2	4880	Max WS	-1.88	11.00	14.87	14.87	0.00	891.37
EAST DETENTION	2	4850	Max WS	-1.92	11.00	14.87	14.87	0.00	2113.88
EAST DETENTION	2	4775	Max WS	6.48	11.00	14.87	14.87	0.01	958.63
EAST DETENTION	2	4653	Max WS	7.42	11.00	14.87	14.87	0.01	549.49
EAST DETENTION	2	4126	Max WS	7.36	11.00	14.87	14.87	0.01	511.14
EAST DETENTION	2	3563	Max WS	10.82	11.00	14.87	14.87	0.02	509.49
EAST DETENTION	2	3562		Culvert					
EAST DETENTION	2	3414	Max WS	8.84	11.00	14.84	14.84	0.02	504.30
EAST DETENTION	2	3335	Max WS	8.84	11.00	14.84	14.84	0.02	461.92
EAST DETENTION	2	3282	Max WS	10.38	11.00	14.84	14.84	0.05	220.17
EAST DETENTION	2	2946	Max WS	16.11	11.00	14.84	14.84	0.08	194.30
EAST DETENTION	2	2945		Culvert					
EAST DETENTION	2	2855	Max WS	15.55	11.00	14.80	14.80	0.07	229.97
EAST DETENTION	2	2804	Max WS	15.55	11.00	14.80	14.80	0.06	255.70
EAST DETENTION	2	1864	Max WS	25.24	11.00	14.80	14.80	0.03	812.18
EAST DETENTION	2	1800	Max WS	25.23	11.00	14.80	14.80	0.02	1322.08
DUMMY	4	1	Max WS	5.00	11.00	14.87	14.87	0.01	334.52
DUMMY	4	0	Max WS	5.00	11.00	14.87	14.87	0.01	336.36

Profile Output Table - ChannelVolume

River	Reach	River Sta	Profile	Volume (acre-ft)
WEST DETENTION	1	5540	Max WS	56.70
WEST DETENTION	1	5460	Max WS	54.28
WEST DETENTION	1	5438	Max WS	52.05
WEST DETENTION	1	5412	Max WS	51.67
WEST DETENTION	1	4950	Max WS	46.15
WEST DETENTION	1	4907	Max WS	45.41
WEST DETENTION	1	4611	Max WS	40.77
WEST DETENTION	1	4537	Max WS	39.82
WEST DETENTION	1	3571	Max WS	28.53
WEST DETENTION	1	3570		Culvert
WEST DETENTION	1	3417	Max WS	26.72
WEST DETENTION	1	3400	Max WS	26.51
WEST DETENTION	1	3383	Max WS	26.33
WEST DETENTION	1	2760	Max WS	19.18
WEST DETENTION	1	2722	Max WS	18.22
WEST DETENTION	1	2635	Max WS	16.39
WEST DETENTION	1	2586	Max WS	14.52
WEST DETENTION	1	2545	Max WS	13.31
WEST DETENTION	1	2507	Max WS	11.68
WEST DETENTION	1	2402	Max WS	9.38
WEST DETENTION	1	2367	Max WS	8.37
WEST DETENTION	1	1987	Max WS	3.90
WEST DETENTION	1	1986		Culvert
WEST DETENTION	1	1922	Max WS	3.16
WEST DETENTION	1	1891	Max WS	2.80
WEST DETENTION	1	1815	Max WS	1.44
SOUTH DETENTION	3	1636	Max WS	91.06
SOUTH DETENTION	3	1561	Max WS	88.93
SOUTH DETENTION	3	1153	Max WS	78.58
SOUTH DETENTION	3	1080	Max WS	76.68
SOUTH DETENTION	3	1012	Max WS	74.62
SOUTH DETENTION	3	945	Max WS	72.52
SOUTH DETENTION	3	942		Inl Struct
SOUTH DETENTION	3	940	Max WS	72.20
SOUTH DETENTION	3	930	Max WS	71.20

SOUTH DETENTION	3	500		Culvert
SOUTH DETENTION	3	150	Max WS	5.87
SOUTH DETENTION	3	100	Max WS	2.54
SOUTH DETENTION	3	90	Max WS	1.87
OFFSITE	R8 NORTH	5350	Max WS	18.01
OFFSITE	R8 NORTH	1100	Max WS	8.25
OFFSITE	R8 NORTH	1050	Max WS	7.99
OFFSITE	R8 NORTH	100	Max WS	0.40
OFFSITE	R8 SOUTH	3500	Max WS	32.09
OFFSITE	R8 SOUTH	3450	Max WS	31.64
OFFSITE	R8 SOUTH	3300		Culvert
OFFSITE	R6 SOUTH	3100	Max WS	28.07
OFFSITE	R8 SOUTH	100	Max WS	
EAST DETENTION	2	5600	Max WS	60.54
EAST DETENTION	2	5560	Max WS	59.35
EAST DETENTION	2	5520	Max WS	57.95
EAST DETENTION	2	5480	Max WS	55.68
EAST DETENTION	2	5455	Max WS	55.23
EAST DETENTION	2	4920	Max WS	48.96
EAST DETENTION	2	4880	Max WS	47.98
EAST DETENTION	2	4850	Max WS	42.23
EAST DETENTION	2	4775	Max WS	35.15
EAST DETENTION	2	4653	Max WS	33.04
EAST DETENTION	2	4126	Max WS	26.62
EAST DETENTION	2	3563	Max WS	20.03
EAST DETENTION	2	3562		Culvert
EAST DETENTION	2	3414	Max WS	18.29
EAST DETENTION	2	3335	Max WS	17.42
EAST DETENTION	2	3282	Max WS	17.00
EAST DETENTION	2	2946	Max WS	15.41
EAST DETENTION	2	2945		Culvert
EAST DETENTION	2	2855	Max WS	14.96
EAST DETENTION	2	2804	Max WS	14.68
EAST DETENTION	2	1864	Max WS	3.16
EAST DETENTION	2	1800	Max WS	1.59
DUMMY	4	1	Max WS	0.10
DUMMY	4	0	Max WS	0.02

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10year 24hour Model

HEC-RAS Version 3.1.3 May 2005
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X   X   XXXXX   XXXX   XXXX   XX   XXXX
X   X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X
XXXXXXXX XXXX   X   XXX XXXX XXXXXXX XXXX
X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
    
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PROJECT DATA

Project Title: GREENBRIAR FARMS
 Project File : GB.prj
 Run Date and Time: 7/15/2005 10:41:24 AM

Project in English units

Project Description:

- References:
 1) Offsite Drainage Improvements-Metro Air Park, Exhibit D-2
 (Offsite improved channels and Interstate 5 crossing)
 2) 100yr-10day &
 10yr-10day, STA 0+00, West Drain Stages, Mark Kubik @ Mead & Hunt Inc.

PLAN DATA

Plan Title: P4-10YR24HR-DEVELOP
 Plan File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.p05

Geometry Title: DEVELOP
 Geometry File : j:\Jobs\1116-GreenbriarFarms\1116-GreenbriarFarms\Civil\Docs\Report\HEC-RAS\GB.g01

Flow Title :
 Flow File :

Plan Summary Information:

Number of: Cross Sections = 207 Multiple Openings = 0
 Culverts = 6 Inline Structures = 1
 Bridges = 0 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

Profile Output Table - Concise Table 1

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	N.S. Elev (ft)	E.G. Elev (ft)	Vel Chnl (ft/s)	Flow Area (sq ft)
WEST DETENTION	1	5540	Max WS	5.83	11.00	13.54	13.54	0.01	950.13
WEST DETENTION	1	5460	Max WS	5.83	11.00	13.54	13.54	0.01	867.68
WEST DETENTION	1	5438	Max WS	5.83	11.00	13.54	13.54	0.01	397.43
WEST DETENTION	1	5412	Max WS	5.83	11.00	13.54	13.54	0.02	333.77
WEST DETENTION	1	4950	Max WS	5.82	11.00	13.54	13.54	0.02	325.08
WEST DETENTION	1	4907	Max WS	5.82	11.00	13.54	13.54	0.01	426.32
WEST DETENTION	1	4611	Max WS	9.27	11.00	13.54	13.54	0.02	418.73
WEST DETENTION	1	4537	Max WS	11.61	11.00	13.54	13.54	0.04	325.44
WEST DETENTION	1	3571	Max WS	11.60	11.00	13.54	13.54	0.04	323.91
WEST DETENTION	1	3570	Culvert						
WEST DETENTION	1	3417	Max WS	11.38	11.00	13.50	13.50	0.04	318.96
WEST DETENTION	1	3400	Max WS	11.38	11.00	13.50	13.50	0.04	318.63
WEST DETENTION	1	3383	Max WS	13.61	11.00	13.50	13.50	0.04	319.72
WEST DETENTION	1	2760	Max WS	15.56	11.00	13.50	13.50	0.05	318.28
WEST DETENTION	1	2722	Max WS	17.94	11.00	13.50	13.50	0.03	578.20
WEST DETENTION	1	2635	Max WS	17.93	11.00	13.50	13.50	0.03	582.88
WEST DETENTION	1	2586	Max WS	17.93	11.00	13.50	13.50	0.02	831.71
WEST DETENTION	1	2545	Max WS	17.93	11.00	13.50	13.50	0.02	832.26

WEST DETENTION	1	2507	Max WS	17.93	11.00	13.50	13.50	0.03	588.23
WEST DETENTION	1	2402	Max WS	17.93	11.00	13.50	13.50	0.03	581.57
WEST DETENTION	1	2367	Max WS	20.91	11.00	13.50	13.50	0.06	325.70
WEST DETENTION	1	1987	Max WS	20.91	11.00	13.50	13.50	0.07	320.13
WEST DETENTION	1	1986		Culvert					
WEST DETENTION	1	1922	Max WS	20.82	11.00	13.41	13.41	0.07	306.61
WEST DETENTION	1	1891	Max WS	20.81	11.00	13.41	13.41	0.07	306.21
WEST DETENTION	1	1815	Max WS	23.06	11.00	13.41	13.41	0.04	656.13
WEST DETENTION	1	1636	Max WS	48.55	11.00	13.41	13.41	0.05	910.82
SOUTH DETENTION	3	1561	Max WS	48.54	11.00	13.41	13.41	0.08	620.72
SOUTH DETENTION	3	1153	Max WS	55.63	11.00	13.41	13.41	0.07	761.75
SOUTH DETENTION	3	1080	Max WS	55.62	11.00	13.41	13.41	0.08	656.09
SOUTH DETENTION	3	1012	Max WS	61.38	11.00	13.41	13.41	0.06	1083.90
SOUTH DETENTION	3	945	Max WS	61.37	11.00	13.41	13.41	0.09	695.48
SOUTH DETENTION	3	942		Inl Struct					
SOUTH DETENTION	3	940	Max WS	61.37	2.70	13.40	13.40	0.02	3772.13
SOUTH DETENTION	3	930	Max WS	61.37	2.70	13.40	13.40	0.02	3772.13
SOUTH DETENTION	3	500		Culvert					
SOUTH DETENTION	3	150	Max WS	61.36	2.20	8.19	8.19	0.04	1710.23
SOUTH DETENTION	3	100	Max WS	61.23	2.20	8.19	8.19	0.04	1710.23
SOUTH DETENTION	3	90	Max WS	61.21	2.20	8.19	8.19	0.04	1710.23
OFFSITE	RB NORTH	5350	Max WS	241.70	9.00	13.95	14.05	2.51	96.17
OFFSITE	RB NORTH	1100	Max WS	229.18	6.00	8.26	8.09	6.36	36.01
OFFSITE	RB NORTH	1050	Max WS	229.88	2.30	8.30	8.32	1.16	197.86
OFFSITE	RB NORTH	100	Max WS	229.04	2.20	8.19	8.21	1.16	196.88
OFFSITE	RB SOUTH	3500	Max WS	290.25	2.20	8.19	8.22	1.47	196.88
OFFSITE	RB SOUTH	3450	Max WS	290.23	2.20	8.20	8.22	1.13	257.17
OFFSITE	RB SOUTH	3300		Culvert					
OFFSITE	RB SOUTH	3100	Max WS	290.09	2.00	7.99	8.01	1.14	255.17
OFFSITE	RB SOUTH	100	Max WS	128.88	1.80	7.56	7.57	0.70	184.16
EAST DETENTION	2	5600	Max WS	-0.83	11.00	13.54	13.54	0.00	950.30
EAST DETENTION	2	5560	Max WS	-0.83	11.00	13.54	13.54	0.00	944.64
EAST DETENTION	2	5520	Max WS	-0.83	11.00	13.54	13.54	0.00	876.14
EAST DETENTION	2	5480	Max WS	-0.84	11.00	13.54	13.54	0.00	395.08
EAST DETENTION	2	5455	Max WS	-0.84	11.00	13.54	13.54	0.00	325.47
EAST DETENTION	2	4920	Max WS	-0.84	11.00	13.54	13.54	0.00	324.97
EAST DETENTION	2	4880	Max WS	-0.85	11.00	13.54	13.54	0.00	569.95
EAST DETENTION	2	4850	Max WS	-0.85	11.00	13.54	13.54	0.00	1374.82
EAST DETENTION	2	4775	Max WS	6.76	11.00	13.54	13.54	0.01	617.66
EAST DETENTION	2	4653	Max WS	8.65	11.00	13.54	13.54	0.02	349.80
EAST DETENTION	2	4126	Max WS	8.64	11.00	13.54	13.54	0.03	325.81
EAST DETENTION	2	3563	Max WS	12.18	11.00	13.54	13.54	0.04	324.71
EAST DETENTION	2	3562		Culvert					
EAST DETENTION	2	3414	Max WS	11.15	11.00	13.50	13.50	0.04	317.97
EAST DETENTION	2	3335	Max WS	11.15	11.00	13.50	13.50	0.04	289.72
EAST DETENTION	2	3282	Max WS	13.33	11.00	13.50	13.50	0.10	132.23
EAST DETENTION	2	2946	Max WS	19.77	11.00	13.50	13.50	0.17	116.24
EAST DETENTION	2	2945		Culvert					
EAST DETENTION	2	2855	Max WS	19.76	11.00	13.41	13.41	0.15	135.99
EAST DETENTION	2	2804	Max WS	19.76	11.00	13.41	13.41	0.13	152.17
EAST DETENTION	2	1864	Max WS	25.50	11.00	13.41	13.41	0.05	505.61
EAST DETENTION	2	1800	Max WS	25.49	11.00	13.41	13.41	0.03	823.79
DUMMY	4	1	Max WS	5.00	11.00	13.54	13.54	0.03	198.21
DUMMY	4	0	Max WS	5.00	11.00	13.54	13.54	0.03	199.73

Profile Output Table - ChannelVolume

River	Reach	River Sta	Profile	Volume (acre-ft)
WEST DETENTION	1	5540	Max WS	36.06
WEST DETENTION	1	5460	Max WS	34.49
WEST DETENTION	1	5438	Max WS	33.06
WEST DETENTION	1	5412	Max WS	32.82
WEST DETENTION	1	4950	Max WS	29.31
WEST DETENTION	1	4907	Max WS	28.84
WEST DETENTION	1	4611	Max WS	25.87
WEST DETENTION	1	4537	Max WS	25.27
WEST DETENTION	1	3571	Max WS	18.07
WEST DETENTION	1	3570		Culvert
WEST DETENTION	1	3417	Max WS	16.92
WEST DETENTION	1	3400	Max WS	16.79
WEST DETENTION	1	3383	Max WS	16.68
WEST DETENTION	1	2760	Max WS	12.17
WEST DETENTION	1	2722	Max WS	11.55
WEST DETENTION	1	2635	Max WS	10.38
WEST DETENTION	1	2586	Max WS	9.18
WEST DETENTION	1	2545	Max WS	8.41
WEST DETENTION	1	2507	Max WS	7.36
WEST DETENTION	1	2402	Max WS	5.88
WEST DETENTION	1	2367	Max WS	5.24
WEST DETENTION	1	1987	Max WS	2.42
WEST DETENTION	1	1986		Culvert
WEST DETENTION	1	1922	Max WS	1.96
WEST DETENTION	1	1891	Max WS	1.74
WEST DETENTION	1	1815	Max WS	0.90
SOUTH DETENTION	3	1636	Max WS	65.25
SOUTH DETENTION	3	1561	Max WS	63.92
SOUTH DETENTION	3	1153	Max WS	57.44
SOUTH DETENTION	3	1080	Max WS	56.26
SOUTH DETENTION	3	1012	Max WS	54.96
SOUTH DETENTION	3	945	Max WS	53.66
SOUTH DETENTION	3	942		Inl Struct
SOUTH DETENTION	3	940	Max WS	53.40

SOUTH DETENTION	3	930	Max WS	52.53
SOUTH DETENTION	3	500	Culvert	
SOUTH DETENTION	3	150	Max WS	3.45
SOUTH DETENTION	3	100	Max WS	1.49
SOUTH DETENTION	3	90	Max WS	1.09
OFFSITE	R8 NORTH	5350	Max WS	12.94
OFFSITE	R8 NORTH	1100	Max WS	4.66
OFFSITE	R8 NORTH	1050	Max WS	4.53
OFFSITE	R8 NORTH	100	Max WS	0.23
OFFSITE	R8 SOUTH	3500	Max WS	17.48
OFFSITE	R8 SOUTH	3450	Max WS	17.22
OFFSITE	R8 SOUTH	3300	Culvert	
OFFSITE	R8 SOUTH	3100	Max WS	15.16
OFFSITE	R8 SOUTH	100	Max WS	
EAST DETENTION	2	5600	Max WS	38.37
EAST DETENTION	2	5560	Max WS	37.60
EAST DETENTION	2	5520	Max WS	36.69
EAST DETENTION	2	5480	Max WS	35.23
EAST DETENTION	2	5455	Max WS	34.94
EAST DETENTION	2	4920	Max WS	30.94
EAST DETENTION	2	4880	Max WS	30.32
EAST DETENTION	2	4850	Max WS	26.60
EAST DETENTION	2	4775	Max WS	22.01
EAST DETENTION	2	4653	Max WS	20.66
EAST DETENTION	2	4126	Max WS	16.56
EAST DETENTION	2	3563	Max WS	12.36
EAST DETENTION	2	3562	Culvert	
EAST DETENTION	2	3414	Max WS	11.26
EAST DETENTION	2	3335	Max WS	10.71
EAST DETENTION	2	3282	Max WS	10.46
EAST DETENTION	2	2946	Max WS	9.50
EAST DETENTION	2	2945	Culvert	
EAST DETENTION	2	2855	Max WS	9.23
EAST DETENTION	2	2804	Max WS	9.06
EAST DETENTION	2	1864	Max WS	1.97
EAST DETENTION	2	1800	Max WS	1.00
DUMMY	4	1	Max WS	0.06
DUMMY	4	0	Max WS	0.01



**8. APPENDIX C –
Existing Culvert Capacities**

AppC.TXT
 BOX CULVERT ANALYSIS
 COMPUTATION OF CULVERT PERFORMANCE CURVE

July 15, 2005

PROGRAM INPUT DATA	
DESCRIPTION	VALUE
Culvert Span (ft).....	8.0
Culvert Rise (ft).....	5.0
FHWA Chart Number.....	8
FHWA Scale Number (Type of Culvert Entrance).....	1
Manning's Roughness Coefficient (n-value).....	0.012
Entrance Loss Coefficient of Culvert Opening.....	0.5
Culvert Length (ft).....	340.0
Invert Elevation at Downstream end of Culvert (ft).....	5.3
Invert Elevation at Upstream end of Culvert (ft).....	5.5
Culvert Slope (ft/ft).....	0.0006
Starting Flow Rate (cfs).....	301.0
Incremental Flow Rate (cfs).....	0.0
Ending Flow Rate (cfs).....	301.0
Starting Tailwater Depth (ft).....	7.0
Incremental Tailwater Depth (ft).....	4.2
Ending Tailwater Depth (ft).....	11.2

COMPUTATION RESULTS

Flow Rate (cfs)	Tailwater Depth (ft)	Headwater Inlet Control (ft)	Headwater Outlet Control (ft)	Normal Depth (ft)	Critical Depth (ft)	Depth at Outlet (ft)	Outlet Velocity (fps)
301.0	7.0	5.73	8.83	5.0	3.53	5.0	7.53
301.0	11.2	5.73	13.03	5.0	3.53	5.0	7.53

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Results and Discussion:

Under existing conditions, the 100-year peak flow through the 3-8'x5' box culverts is 904cfs (see Table 1). The culvert was analyzed individually by prorating the flow.

9. APPENDIX D -
Gate Maximum Outflow Calculations

Worksheet Worksheet for Broad Crested Weir

Project Description	
Worksheet	Weir
Type	Broad Crested Weir
Solve For	Discharge

Input Data	
Headwater Elevation	14.90 ft
Crest Elevation	14.00 ft
Tailwater Elevation	11.20 ft
Crest Surface Type	Paved
Crest Breadth	6.00 ft
Crest Length	16.00 ft

Results	
Discharge	41.69 cfs
Headwater Height Above Crest	0.90 ft
Tailwater Height Above Crest	-2.80 ft
Discharge Coefficient	3.05 US
Submergence Factor	1.00
Adjusted Discharge Coefficient	3.05 US
Flow Area	14.4 ft ²
Velocity	2.90 ft/s
Wetted Perimeter	17.80 ft
Top Width	16.00 ft

Result and Discussion:

During the peak stage, 14.9', in the pond, the Rubicon gate will close to reduce the total outflow. With the top of the gate at elevation 14.0', there will be weir flow above the gate and the total outflow was determined to be 42 cfs, which is less than the total allowable of 62 cfs.



**10. APPENDIX E –
Onsite Drain Pipe Design**

10-YEAR STORM EVENT ONSITE HYDRAULIC DESIGN SUMMARY

HGL CALCULATIONS - 10 YEAR

FROM NODE	TO NODE	Upstream (FROM) Flowline (ft)	Downstream (TO) Flowline (ft)	Diameter (in)	Pipe Area (ft²)	Length (ft)	Design Flow (cfs)	Pipe-full Velocity (ft/s)	Pipe-full Flow (cfs)	HGL Slope (ft/ft)	Head Loss (ft)	Pipe Slope (ft/ft)	US HGL (ft)	US Crown Elev. (ft)	DIS HGL (ft)	DIS Crown Elev. (ft)	Average Pad Elev. (ft)	Average Gutter Elev. (ft)	Average Target HGL Elev. (ft)	0.5ft Freeboard ok?	Target to Calc. Delta (ft)	Pipe Submrg. (ft)	Pipe Cover (ft)	
		10.91	10.43		7.07	0.0015	14.20	2.0	14.20	0.0015	1.22	0.0006	16.33	13.91	15.10	13.43	13.43	18.0	18.0	18.0	ok	1.57	2.42	4.8
		9.43	9.06		12.57	0.0013	25.28	2.0	25.28	0.0013	1.19	0.0004	15.10	13.43	13.92	13.06	13.06	16.0	16.0	16.0	ok	0.90	1.66	3.3
		9.70	9.58		9.62	0.0007	19.35	2.0	19.35	0.0007	0.20	0.0005	14.12	13.06	13.92	13.06	13.06	15.0	15.0	15.0	ok	1.08	0.91	3.6
		9.06	9.00		12.57	0.0030	25.28	2.0	25.28	0.0030	0.42	0.0004	14.92	13.06	13.50	13.00	13.00	17.0	17.0	17.0	ok	0.95	0.86	2.6
		10.76	10.22		9.81	0.0026	9.89	2.0	9.89	0.0026	1.79	0.0008	16.05	13.26	14.26	12.72	12.72	15.0	15.0	15.0	ok	0.74	1.54	4.4
		9.62	9.00		9.82	0.0016	19.35	2.0	19.35	0.0016	0.76	0.0008	14.26	12.72	13.50	12.50	12.50	18.0	18.0	18.0	ok	1.47	2.30	4.5
		11.73	11.04		4.91	0.0012	9.89	2.0	9.89	0.0012	1.59	0.0005	16.53	13.54	14.93	13.19	13.19	17.0	17.0	17.0	ok	2.07	1.40	4.2
		10.04	9.69		9.62	0.0013	25.28	2.0	25.28	0.0013	0.61	0.0004	14.11	13.19	14.11	13.19	13.19	15.0	15.0	15.0	ok	0.89	0.92	2.5
		11.14	10.45		4.91	0.0013	9.89	2.0	9.89	0.0013	2.05	0.0008	14.11	13.19	14.88	12.95	12.95	18.0	18.0	18.0	ok	1.06	3.29	5.1
		8.45	9.00		9.62	0.0015	9.89	2.0	9.89	0.0015	1.39	0.0005	16.94	13.64	14.88	12.95	12.95	16.5	16.5	16.5	ok	1.61	1.94	4.2
		10.83	10.44		4.91	0.0019	9.89	2.0	9.89	0.0019	0.95	0.0008	15.58	13.33	13.50	12.50	12.50	16.0	16.0	16.0	ok	0.42	2.25	3.4
		9.44	9.00		9.62	0.0013	19.35	2.0	19.35	0.0013	1.13	0.0005	14.63	12.94	14.63	12.50	12.50	17.0	17.0	17.0	ok	1.97	1.69	3.6
		10.63	10.25		4.91	0.0028	9.89	2.0	9.89	0.0028	1.28	0.0008	15.62	13.13	14.35	12.75	12.75	15.5	15.5	15.5	ok	1.36	2.49	4.6
		9.25	9.00		9.62	0.0017	19.35	2.0	19.35	0.0017	0.85	0.0005	14.35	12.75	13.50	12.50	12.50	16.0	16.0	16.0	ok	1.15	1.80	3.5
		10.04	9.74		7.07	0.0015	14.20	2.0	14.20	0.0015	0.75	0.0006	15.53	13.04	14.77	12.74	12.74	18.0	18.0	18.0	ok	0.47	2.48	3.7
		9.62	9.00		9.62	0.0026	14.20	2.0	14.20	0.0026	1.27	0.0005	14.77	12.74	13.50	12.50	12.50	15.0	15.0	15.0	ok	0.23	2.04	3.0
		11.63	11.02		7.07	0.0014	14.20	2.0	14.20	0.0014	1.47	0.0006	16.81	14.63	15.34	14.02	14.02	17.5	17.5	17.5	ok	2.69	2.16	5.6
		10.02	9.72		12.57	0.0012	25.28	2.0	25.28	0.0012	0.91	0.0004	15.34	14.02	14.43	13.72	13.72	16.0	16.0	16.0	ok	1.57	1.32	4.2
		11.79	11.30		4.91	0.0018	9.89	2.0	9.89	0.0018	1.22	0.0008	17.18	14.29	15.97	13.80	13.80	18.0	18.0	18.0	ok	1.82	2.90	5.4
		10.30	9.93		12.57	0.0013	9.89	2.0	9.89	0.0013	0.96	0.0005	15.97	13.90	13.50	13.00	13.00	16.0	16.0	16.0	ok	2.03	2.16	4.9
		11.92	11.49		9.43	0.0014	25.28	2.0	25.28	0.0014	1.50	0.0004	15.90	13.43	13.50	13.00	13.00	18.0	18.0	18.0	ok	1.10	1.57	3.3
		9.99	9.68		12.57	0.0011	9.89	2.0	9.89	0.0011	1.88	0.0006	16.90	14.42	15.02	13.68	13.68	17.0	17.0	17.0	ok	1.10	2.48	4.3
		9.18	9.00		15.90	0.0013	31.97	2.0	31.97	0.0013	0.83	0.0004	15.02	13.99	14.18	13.68	13.68	15.0	15.0	15.0	ok	0.92	0.50	2.0
		11.05	10.42		4.91	0.0020	9.89	2.0	9.89	0.0020	2.45	0.0008	14.02	13.68	13.50	13.50	13.50	16.0	16.0	16.0	ok	0.81	0.52	4.10
		9.42	9.00		9.62	0.0030	14.20	2.0	14.20	0.0030	1.69	0.0005	15.19	13.55	15.19	12.92	12.92	18.0	18.0	18.0	ok	0.52	2.27	3.8
		9.39	9.00		7.07	0.0030	14.20	2.0	14.20	0.0030	1.88	0.0006	15.48	12.99	13.50	12.00	12.00	16.0	16.0	16.0	ok	0.95	3.09	4.3
		10.53	10.12		3.14	0.0047	6.31	2.0	6.31	0.0047	1.89	0.0010	15.82	12.53	13.83	12.12	12.12	16.5	16.5	16.5	ok	0.66	3.29	4.7
		9.12	9.00		7.07	0.0022	14.20	2.0	14.20	0.0022	0.43	0.0008	13.93	12.12	13.50	12.00	12.00	15.0	15.0	15.0	ok	1.07	1.61	3.6
		10.12	9.62		4.91	0.0014	9.89	2.0	9.89	0.0014	0.93	0.0008	14.87	12.52	13.93	12.12	12.12	15.0	15.0	15.0	ok	0.72	2.25	3.1
		11.15	10.67		7.07	0.0009	9.89	2.0	9.89	0.0009	0.67	0.0008	14.28	13.65	13.61	13.07	13.07	15.0	15.0	15.0	ok	0.72	0.64	2.1
		10.66	10.07		12.57	0.0014	14.20	2.0	14.20	0.0014	1.33	0.0006	14.94	13.66	13.61	13.07	13.07	14.5	14.5	14.5	ok	0.06	1.28	2.0
		9.07	9.00		4.91	0.0007	25.28	2.0	25.28	0.0007	0.11	0.0004	13.61	13.07	13.50	13.00	13.00	14.5	14.5	14.5	ok	0.89	0.54	2.1
		10.86	10.32		7.07	0.0008	9.89	2.0	9.89	0.0008	0.54	0.0008	14.07	12.82	14.07	12.82	12.82	15.0	15.0	15.0	ok	0.39	1.25	2.3
		9.82	9.67		9.62	0.0010	14.20	2.0	14.20	0.0010	0.24	0.0005	13.83	12.67	13.83	12.67	12.67	15.0	15.0	15.0	ok	1.17	1.25	2.9
		10.96	10.67		3.14	0.0021	6.31	2.0	6.31	0.0021	0.59	0.0010	14.42	12.96	13.83	12.67	12.67	18.5	18.5	18.5	ok	2.08	1.48	4.2

Input Data
Ext. Pipe
Design Flow=Prerated flow from SacCalc peak flow
POT